

# PROJECT DESIGN DATA

#### APPLICABLE CODES:

JURISPICTION: TOWN OF CAPE CHARLES, VIRGIINIA (NORTHAMPTON COUNTY)

2018 VIRGINIA UNIFORM STATEWIDE BUILDING CODE - entire code series 2018 INTERNATIONAL RESIDENTIOAL CODE

2018 INTERNATIONAL BUILDING CODE

2017 NATIONAL ELECTRIC CODE (NEPA 70) 2018 INTERNATIONAL FIRE PREVENTION CODE

2018 INTERNATIONAL FLIEL GAS CODE

2018 INTERNATIONAL MECHANICAL CODE 2018 INTERNATIONAL PLUMBING CODE

#### BUILDING DESIGN DATA:

IF CONSTRUCTED IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS, THE STRUCTURE, INCLUDING DOORS AND WINDOWS, WILL WITHSTAND 120 MPH WIND SPEEDS WITHOUT MAJOR STRUCTURAL DAMAGE OR FAILURE.

THE BUILDER SHALL BE RESPONSIBLE FOR VERIFYING THAT ALL OF THE FOLLOWINGS DESIGN LOAD CRITERIA ARE ADHERED TO IN THE CONSTRUCTION OF THE STRUCTURE.

DESIGN LIVE LOADS:

30 PSF ROOF: 40 PSF FLOORS: BALCONES: 40 PSF 30 PSF SLEEPING AREAS: GROUND SNOW LOAD: 10 PSF WIND LOAD: 120 MPH

HANDRAILS: 200 lbs CONCENTRATED LATERAL LOAD SEISMIC DESIGN CATERGORY:

WIND EXPOSURE: FROST DEPTH: DECAY PROTECTION: SLIGHT TO MODERATE FLOOD-RESISTANT CONSTRUCTION PROVISIONS: NONE

#### <u>INSULATION DATA:</u>

FENESTRATION U-FACTOR 0.32 GLAZED FENESTRATION SHCG

R-15 OR 13 CAVITY + 1 CONT. R-7 + R-13 BATT INSULATION **ALT**: I'' SPRAY-ON FOAM CLOSED CELL, FLASH FOAM R-49 ATTIC/ROOF INSULATION:

149: 7''-8'' SPRAY ON CLOSED CELL FOAM INSULATION ATL: BATT BETWEEN ROOF JOISTS (R-49)

REFER TO TABLE NIIO2.4.1.1 (R4O2.4.1.1)

R-10 CONTINUOUS INSULATION ON INTERIOR CRAWL SPACE WALL: R-13 CAVITY INSULATION ALT:

SLAB ON GRADE: AIR BARRIER & INSULATION:

# PROJECT GENERAL NOTES

#### ALL WORKS SHALL COMPLY WITH ALL APPLICABLE CODES, REGULATIONS AND LOCAL

AMENDMENTS AND INTERPRETATIONS. AL CONSTRUCTIONS MEANS, METHODS, SEQUENCES, PROCEDURES, AD SAFETY PRECAUTIONS ARE THE RESPONSIBILITY OF THE Contractor AND BUILDER.

VERIFICATION OF ALL SITE AND ACTUAL CONDITIONS AND DIMENSIONS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND BUILDER. THE GENERAL NOTES AND TYPICAL CONDITIONS SHALL APPLY OR ALL WORK UNLESS NOTED

OTHERWISE, CONTRACTOR OR BUILDER SHALL PROVIDE SIMILAR QUALITY OF WORK WHERE CONDITIONS ARE NOT SPECIFICALLY DETAILED. CONTRACTOR OR BUILDER SHALL STUDY, COMPARE AND UNDERSTAND ALL DRAWINGS AND SHALL BE RESPONSIBLE FOR COORDINATING WORK BETWEEN ALL CONSULTANTS, TRADES, SUB-CONTRACTORS, AND EMPLOYEES.

FOOTINGS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING CAPACITY OF 2,000PSF. FOOTING SHALL BEAR ON NATURAL, UNDISTURBED SOIL, I'-O'' BELOW ORIGINAL GRADE OR CONTROLLED STRUCTURAL FILL. THE BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2'-6" BELOW FINISHED GRADE, CONTRACTOR TO VERIFY THE ALLOWABLE SOIL PRESSURE IN THE FIELD. IF FOUND TO BE LESS THAN 2,000 PSF, THE FOOTINGS SHALL BE REDESIGNED.

ALL CONCRETE WORK SHALL CONFORM TO THE LATEST APPROVED (BY LOCAL GOVERNMENT) EDITIONS OF THE FOLLOWING A.C.I. AND A.S.T.M. DOCUMENTS: ACI-301 SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS ACI-318 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE

CONCRETE AT 28 DAYS, ALL CONCRETE EXPOSED TO THE WEATHER SHALL BE AIR

<u>REINFORCING STEEL</u> EXCEPT AS NOTED, ALL REINFORCING SHALL BE HIGH STRENGTH NEW BILLET STEEL CONFORMING TO ASTM DESIGNATION A-615 (LATEST LOCAL APPROVED ADDITION) GRADE 60. ALL REINFORCING STEEL SHALL BE DETAILED, FABRICATED, AND PLACED IN ACCORDANCE WITH ACI'S "MANUAL OF STAND PRACTICE FOR DETAILING CONCRETE STRUCTURES'' (ACI-315 - LATEST LOCAL APPROVED EDITION)

ALL CONCRETE, EXCEPT AS NOTED, SHALL BE (F'C=3,000 PSI) STONE AGGREGATE

ALL SPLICES IN REINFORCING SHALL BE CLASS 'B' SPLICES IN ACCORDANCE WITH ACI-318 (LATEST LOCAL APPROVED EDITION) EXCEPT AS NOTED IN PLANS UNLESS OTHERWISE NOTED IN STRUCTURAL DRAWINGS, PROVIDE CONCRETE PROTECTION FOR REINFORCING AS FOLLOWS: CAST AGAINST EARTH - 3''

ALL MASONRY CONSTRUCTION AND MATERIAL USED THEREIN (CONCRETE MASONRY, CLAY MASONRY, MORTAR, GROUT, AND STEEL REINFORCEMENT) SHALL CONFORM TO "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES! (ACI 530-92 / ASCE 5-92 / TMS 402-92) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530,1-92 / ASCE 6-92 / TMS 602-92) IN ALL RESPECTS

UNLESS OTHERWISE NOTED CONCRETE MASONRY UNITS SHALL HAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI MASONRY BEARING WALLS SHALL CONSIST OF STANDARD HOLLOW UNITS CONFORMING TO

CONFORMING TO ASTM C 145. ALL MORTAR SHALL CONFORM TO THE REQUIREMENTS FOR PROPORTIONS, MIXING, STRENGTH, SAMPLING, TESTING AND APPLICATION FOR PORTLAND CEMENT / LIME TYPE 'S'

MORTAR AS DESCRIBED IN ACI 530-92 ALL SOLID CMU IS TO BE 100% SOLID CMU OR HOLLOW CMU WITH ALL CELLS FILLED 100% SOLID WITH PEA GRAVEL CONCRETE WITH F'C=3000 PSI OR GROUT CONFORMING 10 ASTM C 476

PROVIDE 100% SOLID MASONRY BELOW ALL JOIST (WHERE APPLICABLE) OR SLAB BEARING LINES, PROVIDE 16'' HIGH AND 16'' LONG 100% SOLID MASONRY BELOW ALL LINTELS AND BEAMS UNLESS NOTED OTHERWISE ALL MASONRY SHALL BE REINFORCED WITH NO. 9 GAUGE TRUSS TYPE GALVANIZED DUR-O-

WALL SPACED VERTICALLY @ 16" O.C. UNLESS NOTED OTHERWISE, LAP ALL DUR-O-WALL 6" MIN. PROVIDE CORNER AND 1 PIECES AT ALL INTERSECTIONS. PROVIDE SOLID BLOCK OR FILL WALL SOLID WITH AROUT DIRECTLY BELOW ALL CHANGES IN WALL THICKNESS OR CONSTRUCTION AS REQUIRED TO PROVIDE CONTINUOUS BEARING FOR

ALL FACE SHELL OF BLOCK. LOOSE LINETS FOR MASONRY WALL SHALL BE FOR EACH 4" WIDTH OF MASONRY, ONE

STEEL ANGLE AS FOLLOWS: a. 0'-0" 10 3'-0" 3-1/2" x 3-1/2" x 5/16" 3'-|'' 10 5'-0'' 4" x 3-1/2" x 5/16" c. 5'-1" 10 6'-6" 5" x 3-1/2" x 3/8"

d. 6'-7" 10 8'-0" 6" x3/12" x3/8" " ALL ANGLES SHALL HAVE THEIR SHORT LEG OUTSTANDING AND 6" MIN. BEARING STRUCTURAL STEEL ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM SPECIFICATIONS A-36 (LATEST LOCAL

APPROVED) ALL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE AISC MANUAL, AISC SPECIFICATIONS, AND AISC CODE OF STANDARD PRACTICE. ALL STEEL PIPE SHALL CONFORM TO ASTM A-501 , F'Y=36000 PSI OR ASTM A-53 GRADE B, F'Y=35000 PSI. ALL WELDED CONNECTIONS SHALL BE DONE WITH E7OXX ELECTRODES, SHOP AND FIELD WELDS SHALL BE MADE BY APPROVED CERTIFIED WELDERS AND SHALL CONFORM TO THE

FULL STRENGTH OF MATERIALS BEING WELDED UNLESS OTHERWISE NOTED.

STRUCTURAL SOLID WOOD RAFTERS, JOISTS, BEAMS, AND STUDS SHALL BE HEM FIR #2 SURFACE DRY AT A MAXIMUM OF 19% MOISTURE CONTENT, ALL WOOD POSTS 6x6 AND AREATER SHALL BE HEM FIR #1. ALL FABRICATION, ERECTION, OTHER PROCEDURES, AND MINIMUM UNIT STRESSES SHALL CONFORM TO THE CURRENT ''NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION''

WOOD TRUSSES SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE DESIGN SPECIFICATION FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES AND BRACING OF WOOD TRUSSES, COMMENTARY AND RECOMMENDATIONS HIB-91 AS PUBLISHED BY THE

DESIGN SPECIFICATION FOR WOOD CONSTRUCTION. WOOD TRUSSES AND WOOD TRUSS JOISTS ARE TO BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF CONSTRUCTION, SIGNED AND SEALED CALCULATIONS SHALL BE SUBMITTED FOR RECORD.

ALL LAMINATED VENEER LUMBER (LVL) OR PARALLEL STRAND LUMBER (PSL) SHALL BE BY TRUS JOIST MACMILLON (OR APPROVED EQUAL) AND SHALL CONFORM TO THE DATA OF TRUS JOIST MACMILLON. CONTRACTOR SHALL CONFORM TO THE MANUFACTURERS PRINTED INSTALL, BRACE, AND ANCHOR LVL'S IN ACCORDANCE WITH THE MANUFACTURERS PRINTED

INSTRUCTIONS. PROVIDE DOUBLE JOISTS AT PARALLEL PARTITIONS WHERE PARTITION LENGTH EXCEEDS 1/3 JOIST SPAN.

NAILING OF ALL FRAMING SHALL BE SPECIFIED IN THE CONTRACT DOCUMENTS NUT IN NO CASE SHALL BE LESS THAN THE RECOMMENDED NAILING SCHEDULE CONTAINED IN THE 1995 CABO CODE. PROVIDE ONE ROW OF BRIDGING BETWEEN ALL FLOOR AND ROOF JOISTS FOR EACH

8'-0'' OF SPAN. PROVIDE SOLID BRIDGING OR A CONTINUOUS HEADER AT THE BEARING OF ROOF AND FLOOR JOIST ON WOOD PLATES. ALL INTERIOR AND EXTERIOR STUD BEARING WALLS SHALL BE 2x4 OR 2x6 @16" O.C. (SEE PLANS) UNLESS NOTED OTHERWISE. PROVIDE SOLID BRIDGING AT MID-HEIGHT OF ALL STUD WALLS TO 9'-O'' HIGH UNLESS NOTED OTHERWISE, STUD WALLS OVER 9'-O'' SHALL BE

HEIGHT UNLESS NOTED OTHERWISE. PROVIDE DOUBLE STUDS AT ALL CORNERS, SIDES OF ALL OPENINGS, WINDOWS AND DOORS, AND BENEATH ALL WOOD BEAMS AND LINTELS UNLESS NOTED OTHERWISE ON THE PLANS, WOOD BEAMS AND LINTELS SHALL BEAR THE FULL DEPTH OF MULTIPLE STUDS OR POSTS. MULTIPLES STUDS OR POSTS BENEATH WOOD BEAMS AND LINTELS SHALL BE CARRIED THROUGH ANY INTERMEDIATE FLOOR FRAMING TO THE TOP OF FOOTINGS OR

PROVIDED WITH 2 ROWS OF SOLID BRIDGING LOCATED AT THIRD POINTS OF THE STUD

MASONRY FOUNDATION WALLS. ALL MULTIPLE STUDS OR POSTS SHALL BE BLOCKED AT ALL INTERSECTIONS WITH FLOORS AS REQUIRED TO PROVIDE CONTINUOUS SUPPORT TO TOP OF FOUNDATION WALLS. ALL FLUSH JOIST TO BEAM OR BEAM TO BEAM CONNECTIONS SHALL BE MADE WITH JOIST OR BEAM HANGERS TO SUPPORT THE FULL CAPACITY OF THE JOIST OR BEAM.

DRAWING INDEX					
SHEET NUMBER	SHEET NAME	F1851 155UE	CURRENT REVISION	CURRENT REV DATE	
A-001	COVER SHEET	08/14/2023			
A-100	SITE PLAN	08/14/2023			
A-IIO	FLOOR PLAN	08/14/2023			
A-	REFLECTED CEILING, POWER & LIGHTING PLANS	08/14/2023			
A-112	ROOF PLAN	08/14/2023			
A-201	ELEVATIONS	08/14/2023			
A-202	ELEVATIONS ELEVATIONS	08/14/2023			
A-301	BUILDING SECTIONS	08/14/2023			
A-302	BUILDING SECTIONS	08/14/2023			
A-303	BUILDING SECTIONS	08/14/2023			
A-304	BUILDING SECTIONS	08/14/2023			
A-401	WALL SECTIONS	08/14/2023			
A-402	WALL SECTIONS - GARAGE	08/14/2023			
A-403	WALL SECTIONS	08/14/2023			
A-404	WALL SECTIONS	08/14/2023			
A-501	MILLWORK ELEVATIONS - KITCHEN	08/14/2023			

# architects

6278 N FEDERAL HWY #1 FORT LAUDERDALE, FLORIDA 33308

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INKBERRY

**HOLLIES** 

OWNER: TERRY INDUSTRIES

2509 GEORGE MASON DR., #6894 VIRGINIA BEACH, VIRGINIA 23456

SHEET TITLE: **COVER SHEET** 



DRAWN BY:	JFH
REVIEWED BY:	JFH
PROJECT NO:	22-007
DATE:	07/31/2023
SHEET NO.	

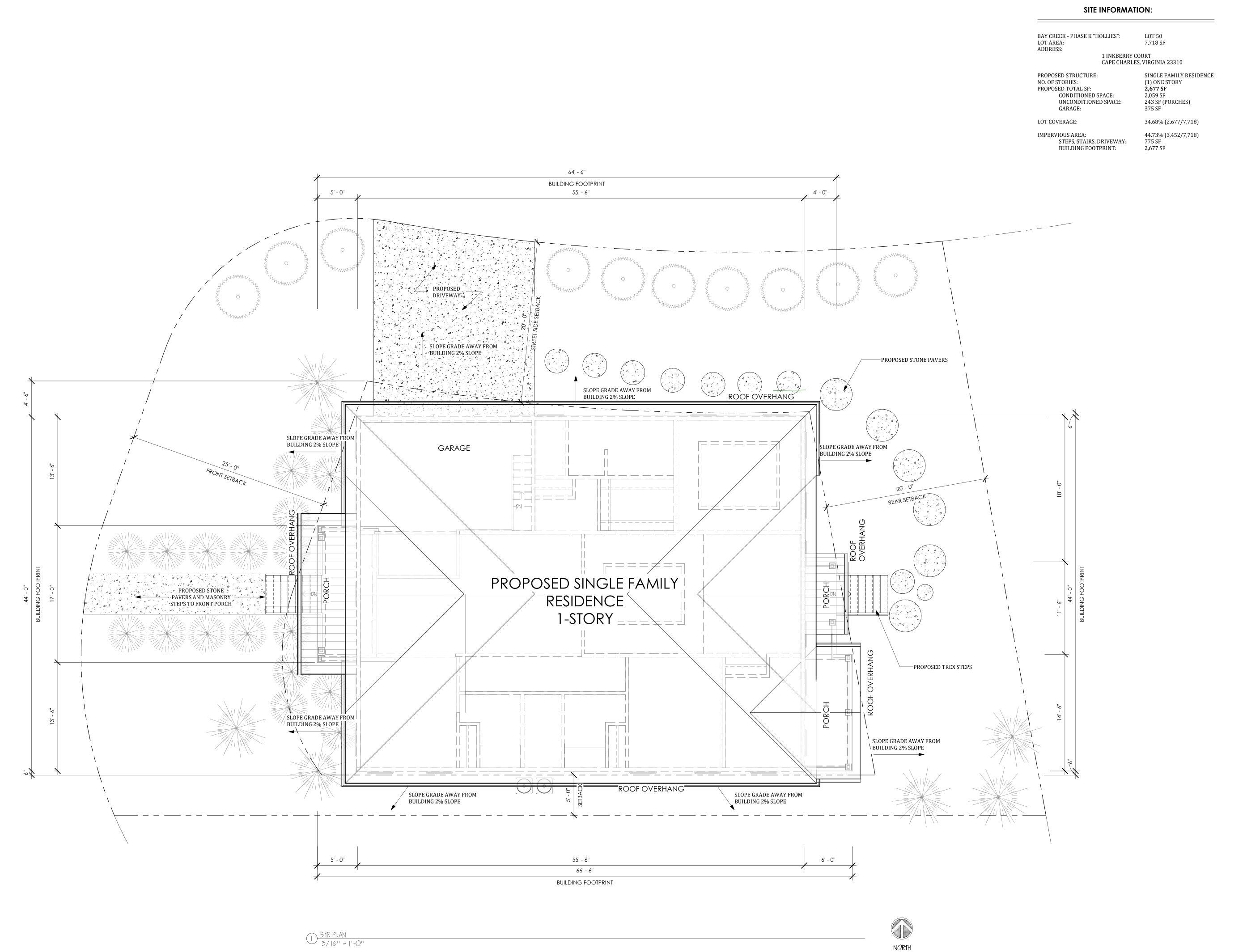
EXTERIOR 2X4 WALL CONSTRUCTION:

FLOOR INSULATION OVER UNCONDITIONED SPACE:

BASEMENT WALL: NOT APPLICABLE

R-10, 2 FT (SLAB EDGE DEPTH)

TO COMPLY WITH NIIO2.4



HARRISSMITH architects

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OWNER:
TERRY INDUSTRIES

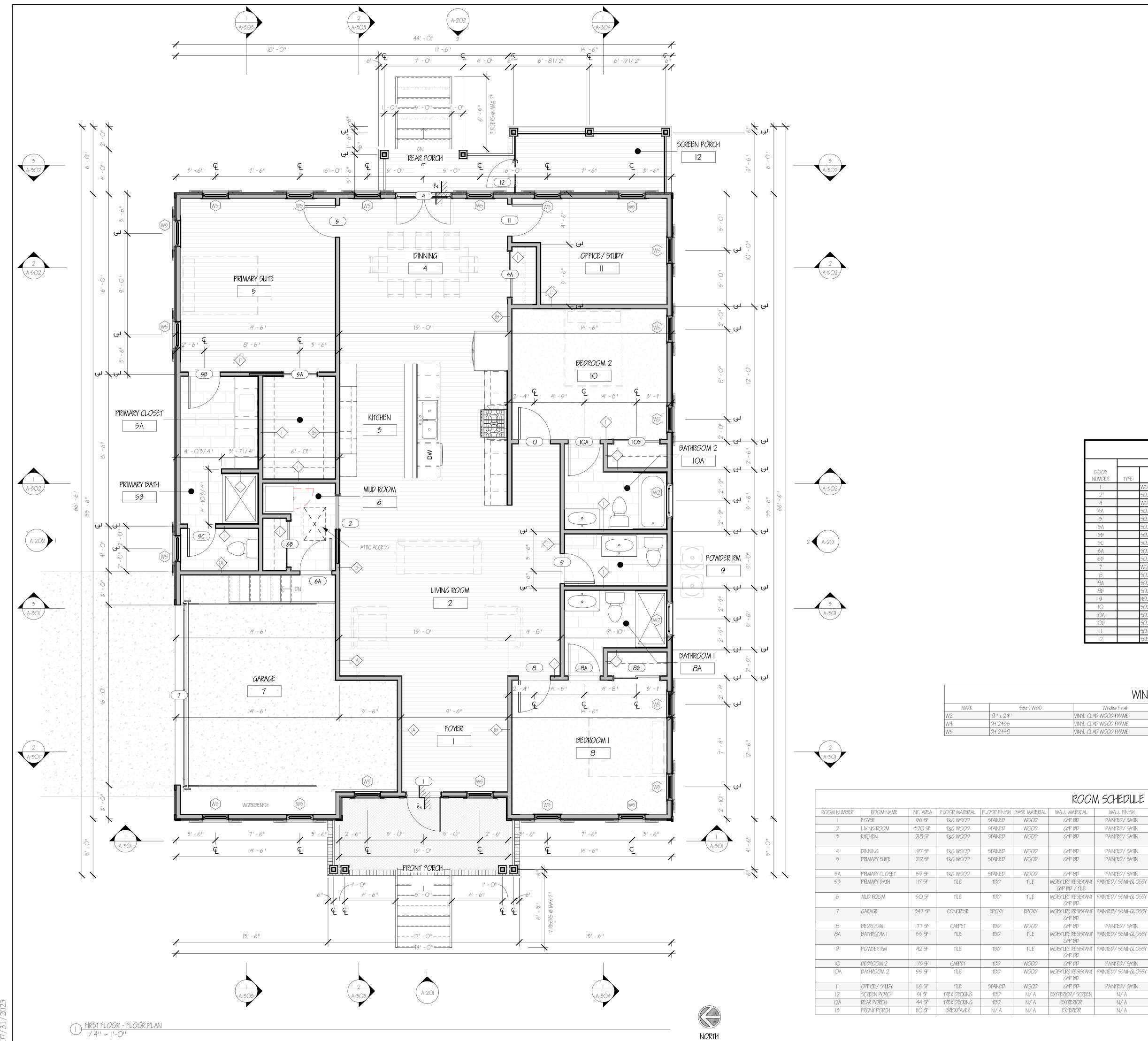
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SHEET TITLE:
SITE PLAN



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A-100



# FLOOR PLAN LEGEND

WALL - PARTITION TAG (SEE SCHEDULE)

DOOR TAG (SEE SCHEDULE)

WINDOW TAG (SEE SCHEDULE)





- I. ALL DIMENSIONS ARE TO FACE OF STUD (UNLESS OTHERWISE NOTED).
- CONTRACTOR TO PROVIDE BACKING AT ALL MILLWORK, CABINET LOCATIONS, AND AT AREAS WHERE FIXTURES AND EQUIPMENT ARE TO BE MOUNTED.
- 3. USE MOISTURE RESISTANT GYPSUM BOARDS IN ALL AREAS WITH HIGH MOISTURE
- 4. USE DUROCK CEMENT BOARDS FOR INTERIOR AREAS THAT WILL HAVE TILE FINISHES. USE BEHIND ALL SHOWER/ TUB ENCLOSURES AND WET WALLS
- SEE TYPICAL WALL SECTIONS FOR ADDITIONAL INFORMATION ON PROPOSED WALL PARTITIONS AND FINISHES.

DOOR SCHEDULE						
			DOOR			
DOOR				NOMINAL SIZE		
NUMBER	TYPE	MATERIAL	WIDTH	HEIGHT	1HICKNESS	REMARKS
		WOOD/GLASS	3' - 0''	6' - 8''	13/4"	
2		SOLID WOOD	3' - 0"	6' - 8''	13/4"	
4		WOOD/GLASS	5' - 0"	6' - 8''	13/4"	
4A		SOLID WOOD	4' - 0"	6' - 8''	13/4"	
5		SOLID WOOD	2' - 10''	6' - 8''	13/4"	
5A		SOLID WOOD	3' - O''	6' - 8''	13/4"	
5B		SOLID WOOD	2' - 10''	6' - 8''	13/4"	
5C		SOLID WOOD	2' - 6"	6' - 8''	1-3/4"	
6A		SOLID WOOD	2' - 10''	6' - 8''	13/4"	
6B		SOLID WOOD	4' - 0''	6' - 8''	13/4"	
7		WOOD/GLASS	16' - 0''	10' - 0''	13/4"	GARAGE DOOR
8		SOLID WOOD	2' - 10''	6' - 8''	13/4"	
8A		SOLID WOOD	2' - 10''	6' - 8''	13/4"	
8B		SOLID WOOD	4' - 0''	6' - 8''	13/4"	
9		HOLLOW CORE WOOD	2' - 8''	6' - 8''	13/4"	
10		SOLID WOOD	2' - 10''	6' - 8''	13/4"	
IOA		SOLID WOOD	2' - 10''	6' - 8''	13/4"	
IOB		SOLID WOOD	4' - 0''	6' - 8''	13/4"	
		SOLID WOOD	2' - 10''	6' - 8''	13/4"	
12		SCREEN DOOR	2' - 6"	6' - 8''	13/4"	OPTIONAL SCREEN ENCLOSURE 113D BY OWNE

WINDOW SCHEDULE						
MARK	Size (WxH)	Window Finish	Glazing	Manufacturer	Sill Height	Comments
W2	18" x 24"	VINYL CLAD WOOD FRAME	CLEAR	Andersen Corporation	5' - 215/16"	ANDERSON SERIES 400
W4	DH 2436	VINYL CLAD WOOD FRAME	CLEAR	Andersen Corporation	3' - 6"	ANDERSON SERIES 400
W5	DH 2448	VINIM CLAD WOOD FRAME	CLEAR	Andersen Cornoration	2' - 0"	ANDERSON SERIES 400

	ROOM JUILDULL												
ROOM NUMBER	ROOM NAME	INT. AREA	FLOOR MATERIAL	FLOOR FINISH	BASE MATERIAL	WALL MATERIAL	WALL FINISH	WALL COLOR	CEILING MATERIAL	CEILING FINISH	CEILING COLOR	CEILING HT	NOTES
I	FOYER	96 SF	1&G WOOD	STAINED	WOOD	GYP BD	PAINTED/SATIN	11315	GYP BD	PAINTED/FLAT	11310	VAULTED	
2	LIVING ROOM	320 SF	1&G WOOD	STAINED	WOOD	GYP BD	PAINTED/SATIN	11310	GYP BD	PAINTED/FLAT	130	VAULTED	
3	KITCHEN	218 54	1&G WOOD	STAINED	WOOD	GYP BD	PAINTED/SATIN	11317	EX	PAINTED/FLAT	11310	10'-0"	OPTION TO PROVIDE COVE CEILING
4	DINNING	197 SF	1&G WOOD	STAINED	WOOD	GYP BD	PAINTED/SATIN	11310	GYP BD	PAINTED/FLAT	130	VAULTED	
5	PRIMARY SUITE	212 54	1&G WOOD	STAINED	WOOD	GYP BD	PAINTED/SATIN	11317	GYP BD	PAINTED/FLAT	130	9'-0"	OPTION TO PROVIDE COVE CEILING
5A	PRIMARY CLOSET	59 SF	1&G WOOD	STAINED	WOOD	GYP BD	PAINTED/SATIN	11310	GYP BD	PAINTED/FLAT	11310	9'-0"	
5B	PRIMARY BATH	117 54	TILE	11317	11LE	MOISTURE RESISTANT GYP BD / TILE	PAINTED/SEMI-GLOSSY	11317	MOISTURE RESISTANT  GYP BD	PAINTED/FLAT	11310	9'-0"	
6	MUD ROOM	50 SF	11LE	11317	11LE	MOISTURE RESISTANT GYP BD	PAINTED/SEMI-GLOSSY	11317	GYP BD	PAINTED/FLAT	130	9'-0"	
7	GARAGE	347 SF	CONCRETE	EPOXY	EPOXY	MOISTURE RESISTANT GYP BD	PAINTED/SEMI-GLOSSY	11317	MOISTURE RESISTANT GYP BD	PAINTED/FLAT	130	12'-0"	
8	BEDROOM I	177 SF	CARPET	11310	WOOD	GYP BD	PAINTED/SATIN	11310	ΕX	PAINTED/FLAT	11310	EX	
8A	BATHROOM I	55 SF	11LE	130	11LE	MOISTURE RESISTANT GYP BD	PAINTED/SEMI-GLOSSY	11317	GYP BD	PAINTED/FLAT	130		
9	POWDER RM	42 SF	11LE	11317	11LE	MOISTURE RESISTANT GYP BD	PAINTED/SEMI-GLOSSY	11317	EX	PAINTED/FLAT	11310	EX	
10	BEDROOM 2	173 54	CARPET	130	WOOD	GYP BD	PAINTED/SATIN	11310	EX	PAINTED/FLAT	130	EX	
IOA	BATHROOM 2	55 SF	11LE	11317	WOOD	MOISTURE RESISTANT GYP BD	PAINTED/SEMI-GLOSSY	11317	GYP BD	PAINTED/FLAT	11310	EX	
II	OFFICE/STUDY	116 SF	TILE	STAINED	WOOD	GYP BD	PAINTED/SATIN	11310	GYP BD	PAINTED/FLAT	130	8'-6"	
12	SCREEN PORCH	5154	TREX DECKING	11310	N/A	EXTRERIOR/SCREEN	N/A			1&G WOOD	130		
12A	REAR PORCH	44 SF	TREX DECKING	11315	N/A	EXTRERIOR	N/A			1&G WOOD	130		
13	FRONT PORCH	110 \$	BRICKPAVER	N/A	N/A	EXTERIOR	N/A	130	GYP BD	1&G WOOD	130	EX	

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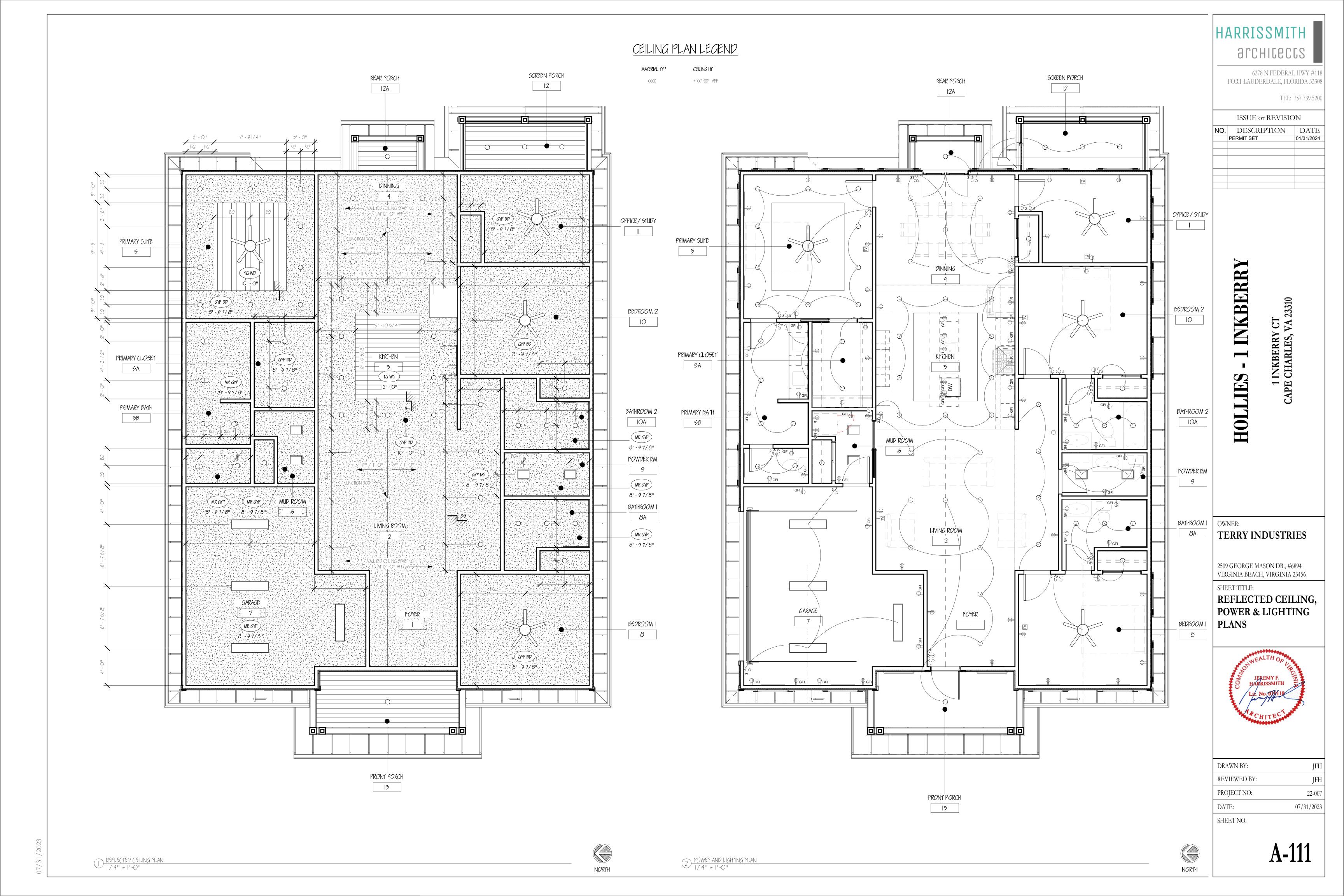
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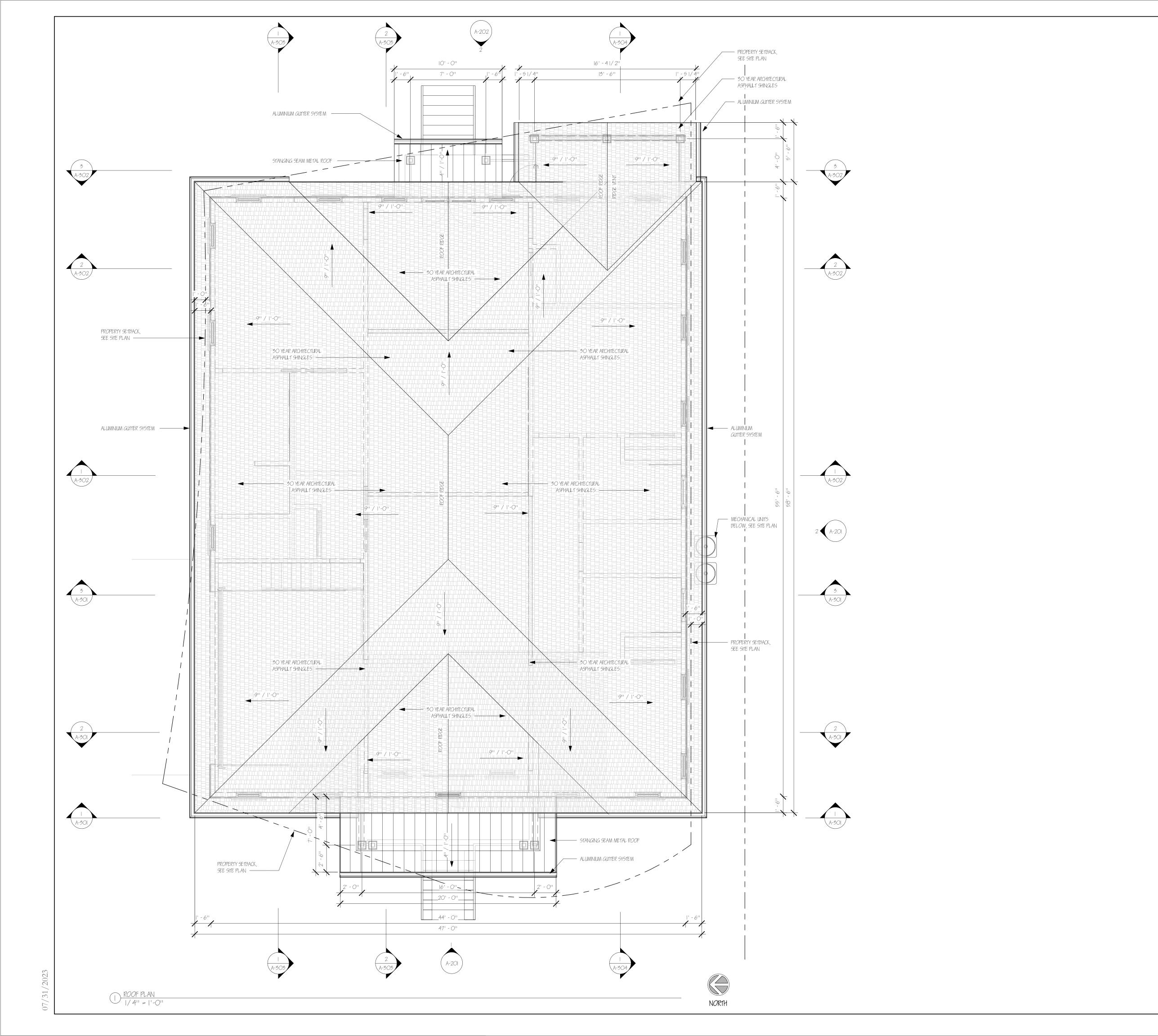
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SHEET TITLE: FLOOR PLAN



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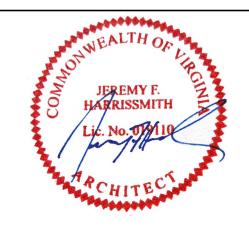
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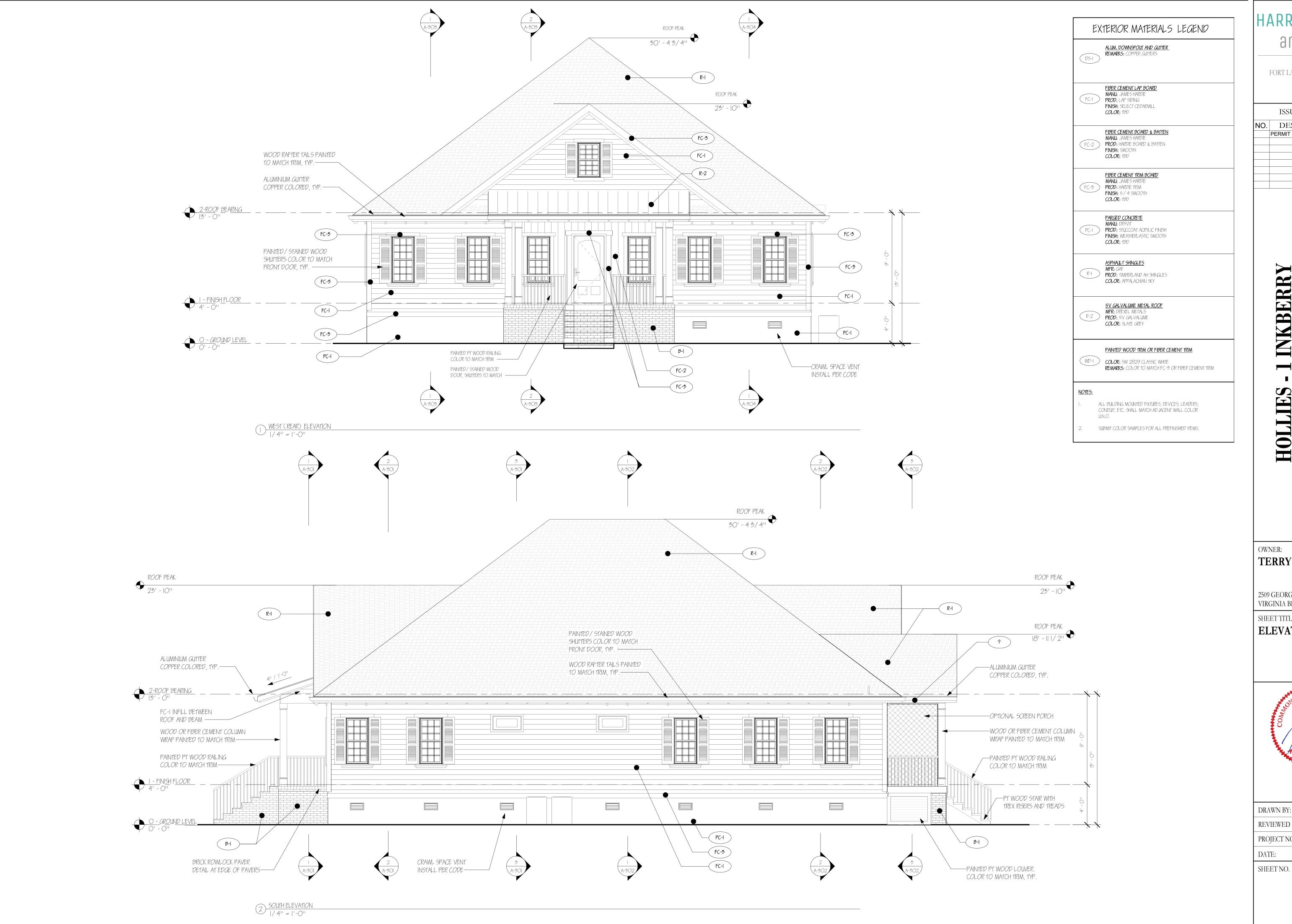
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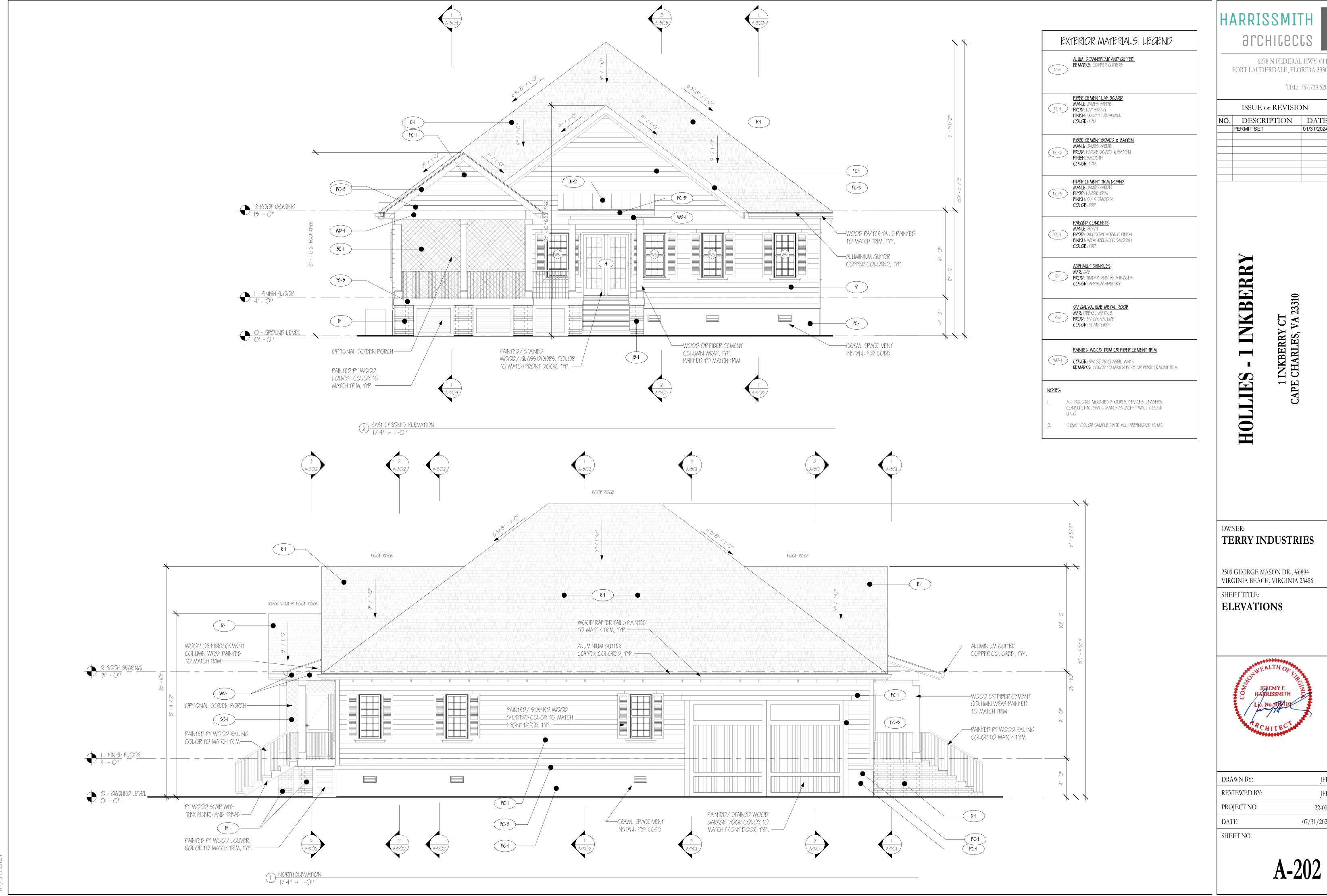
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SHEET TITLE: **ELEVATIONS** 



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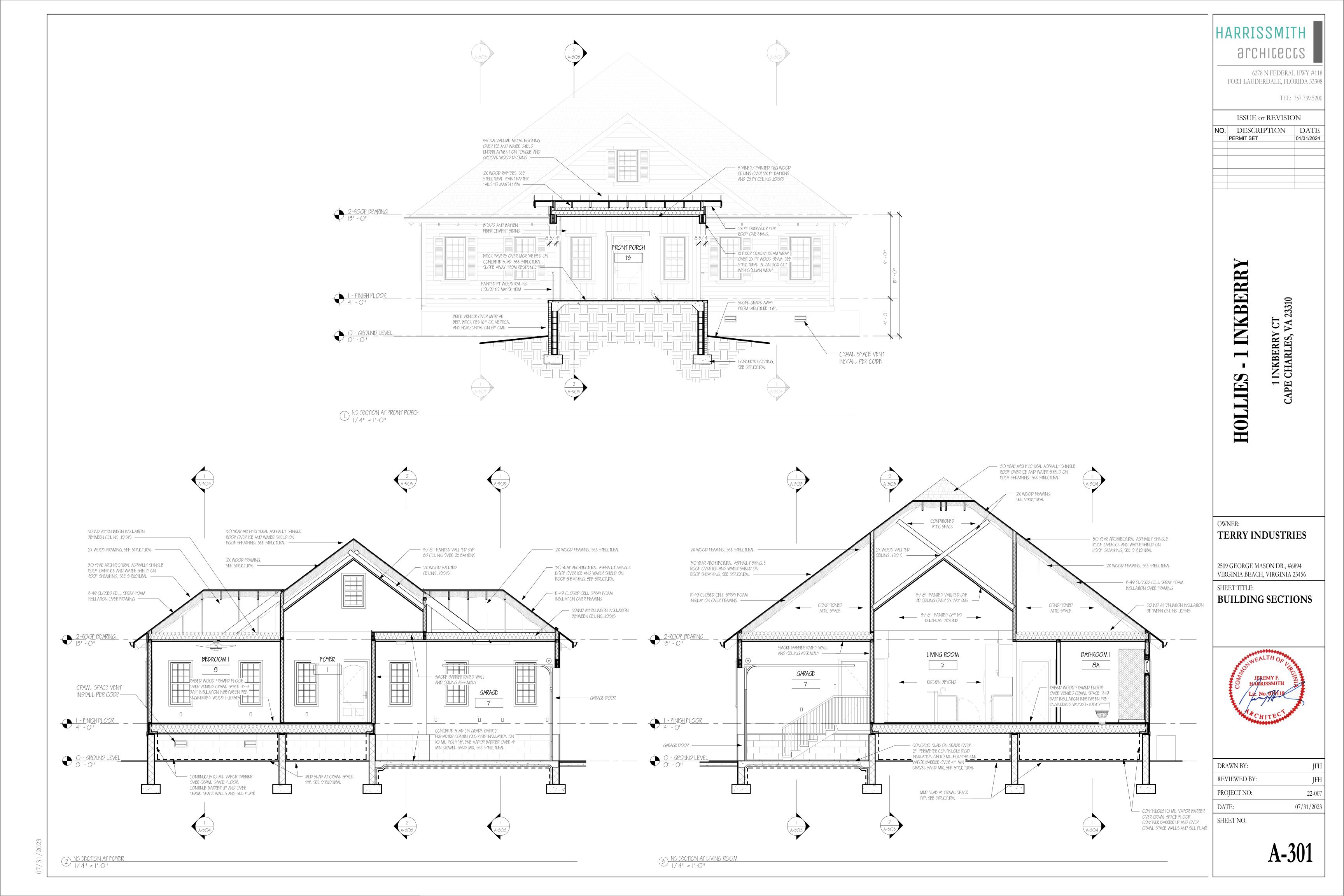
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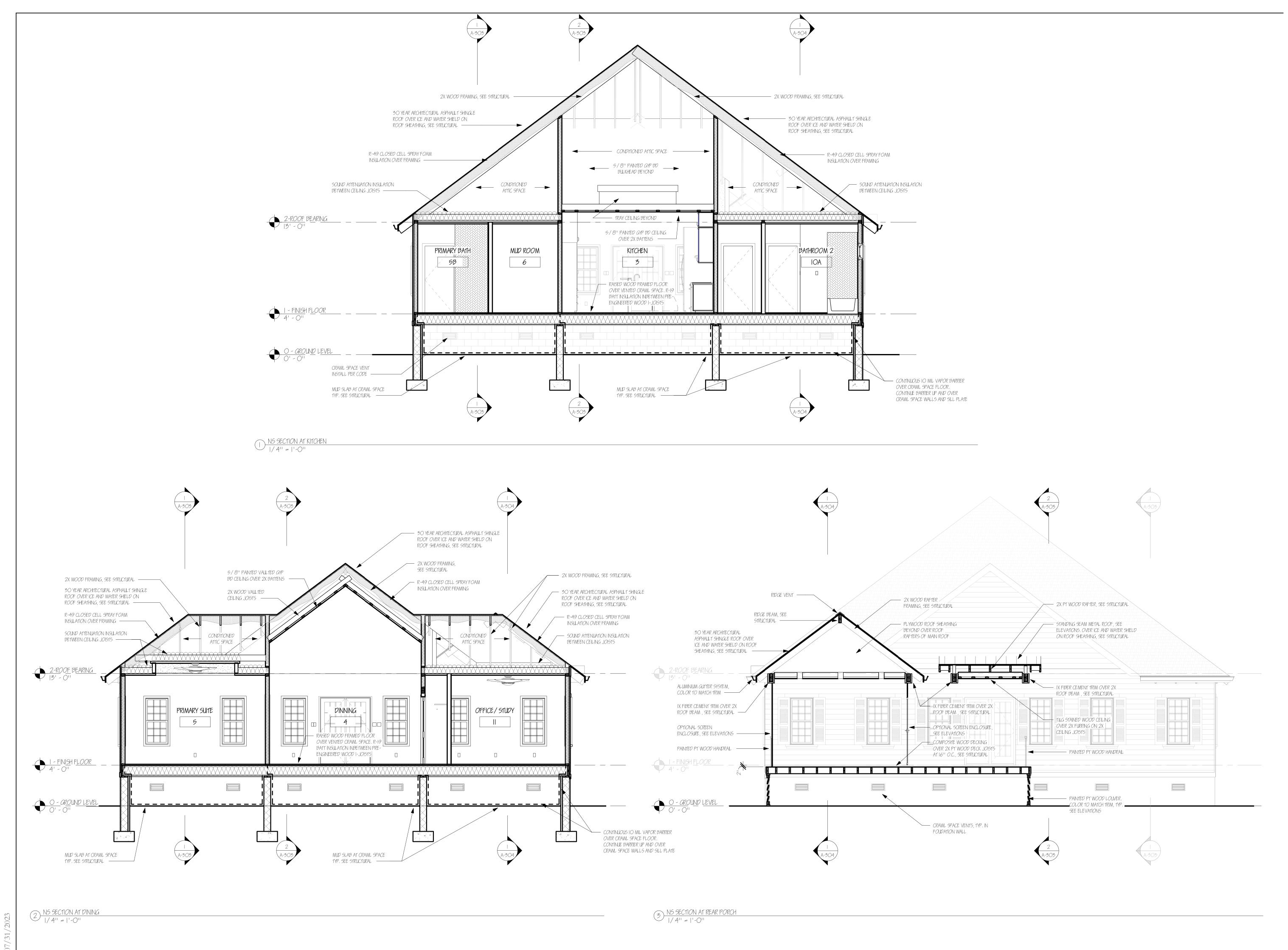
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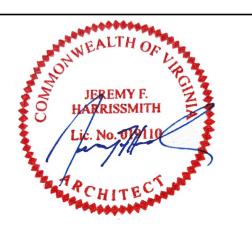
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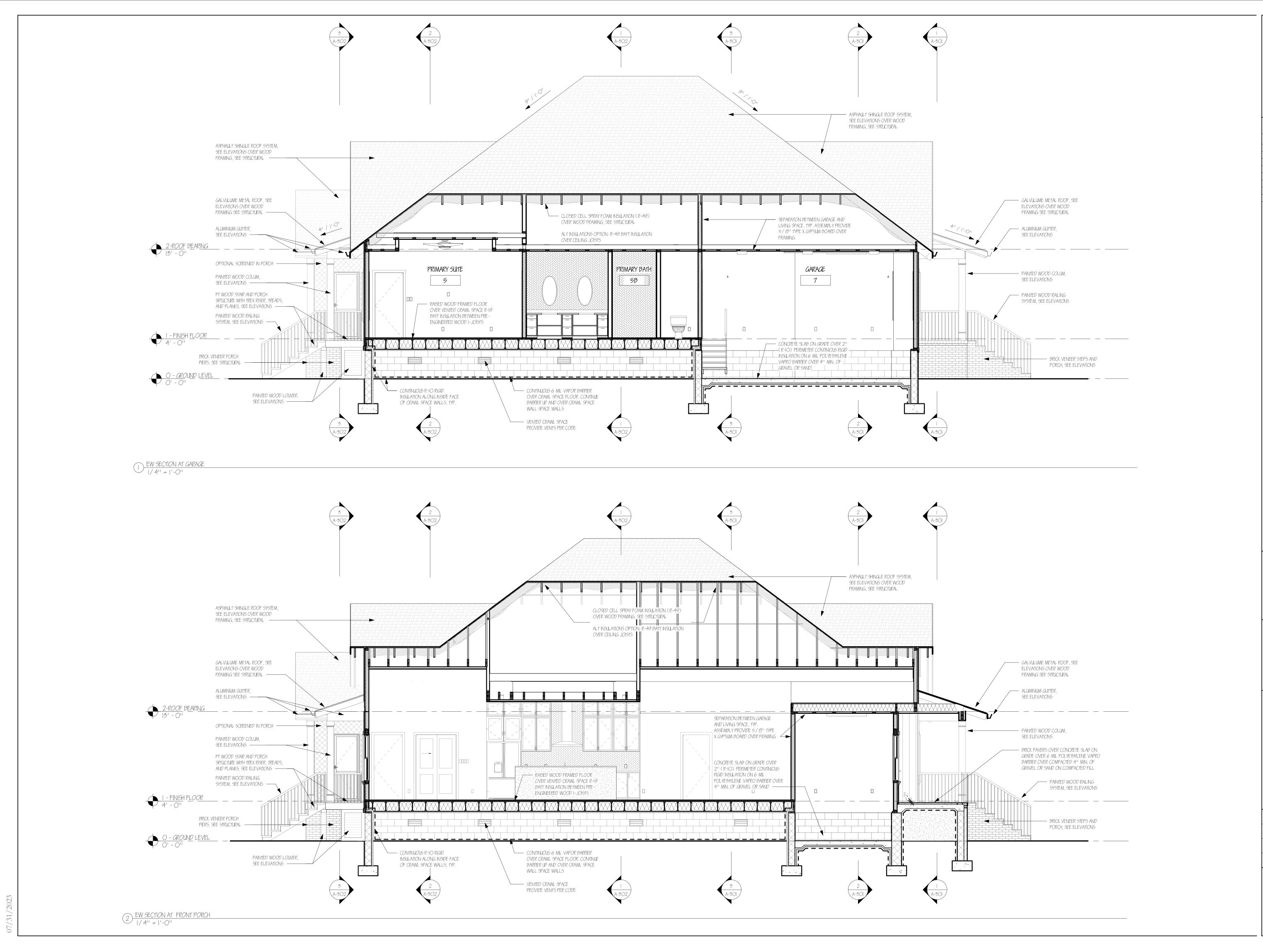
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SHEET TITLE: **BUILDING SECTIONS** 



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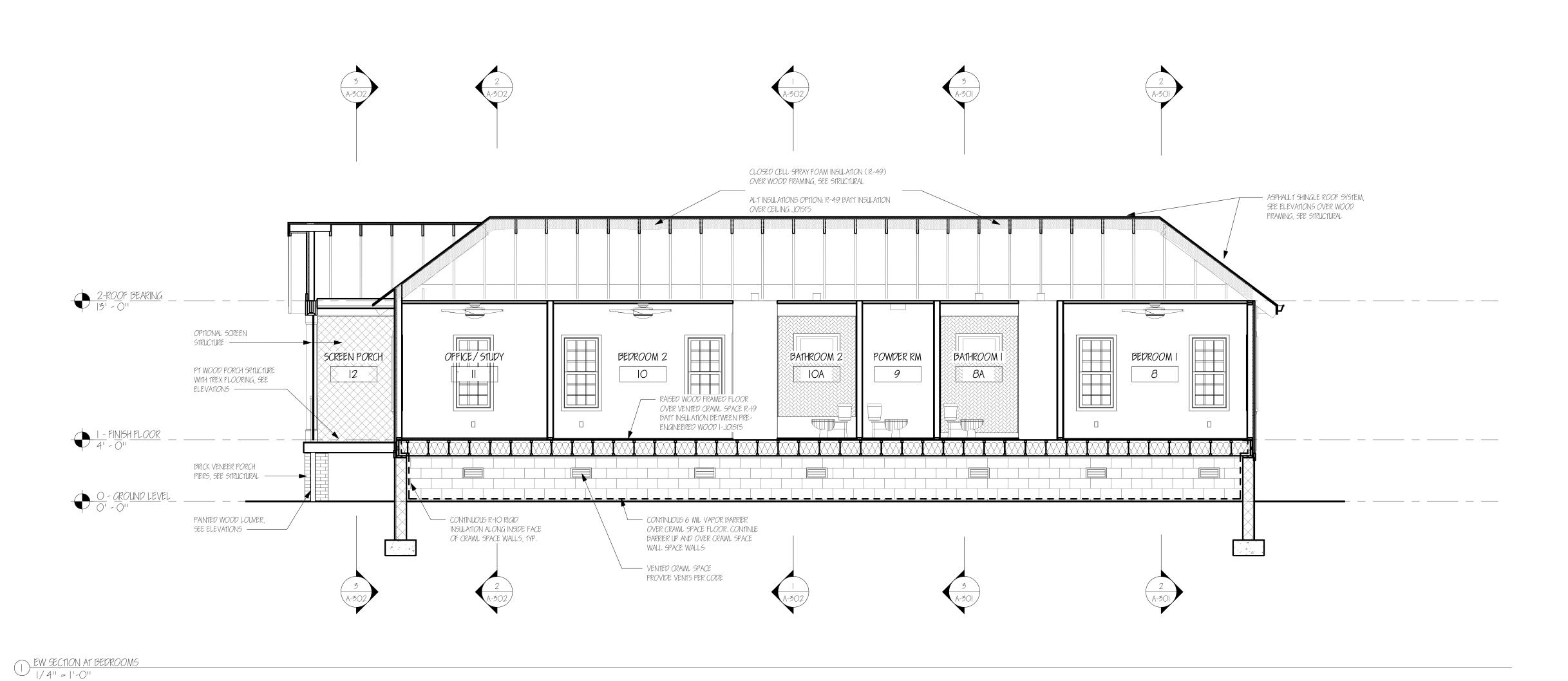
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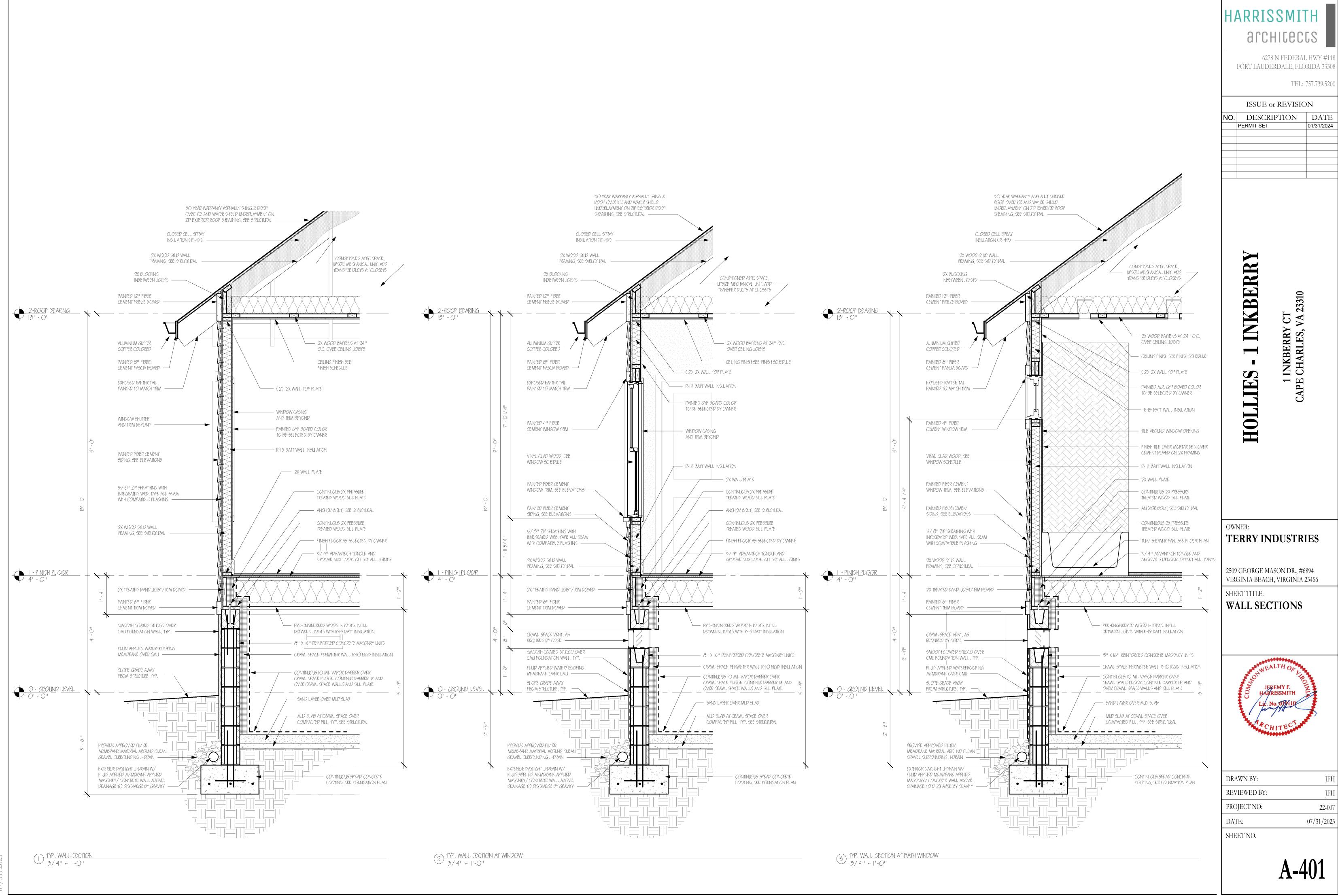
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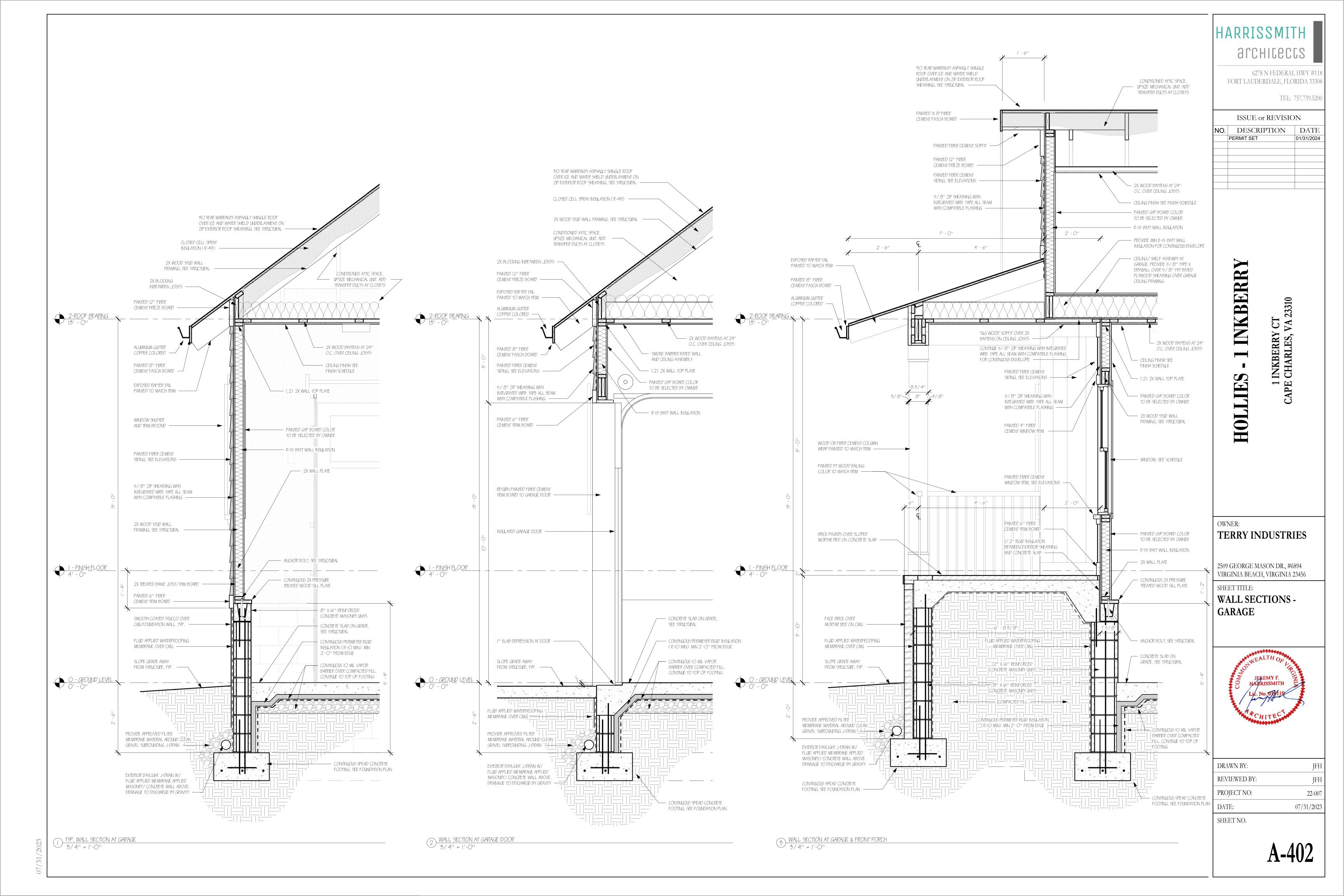


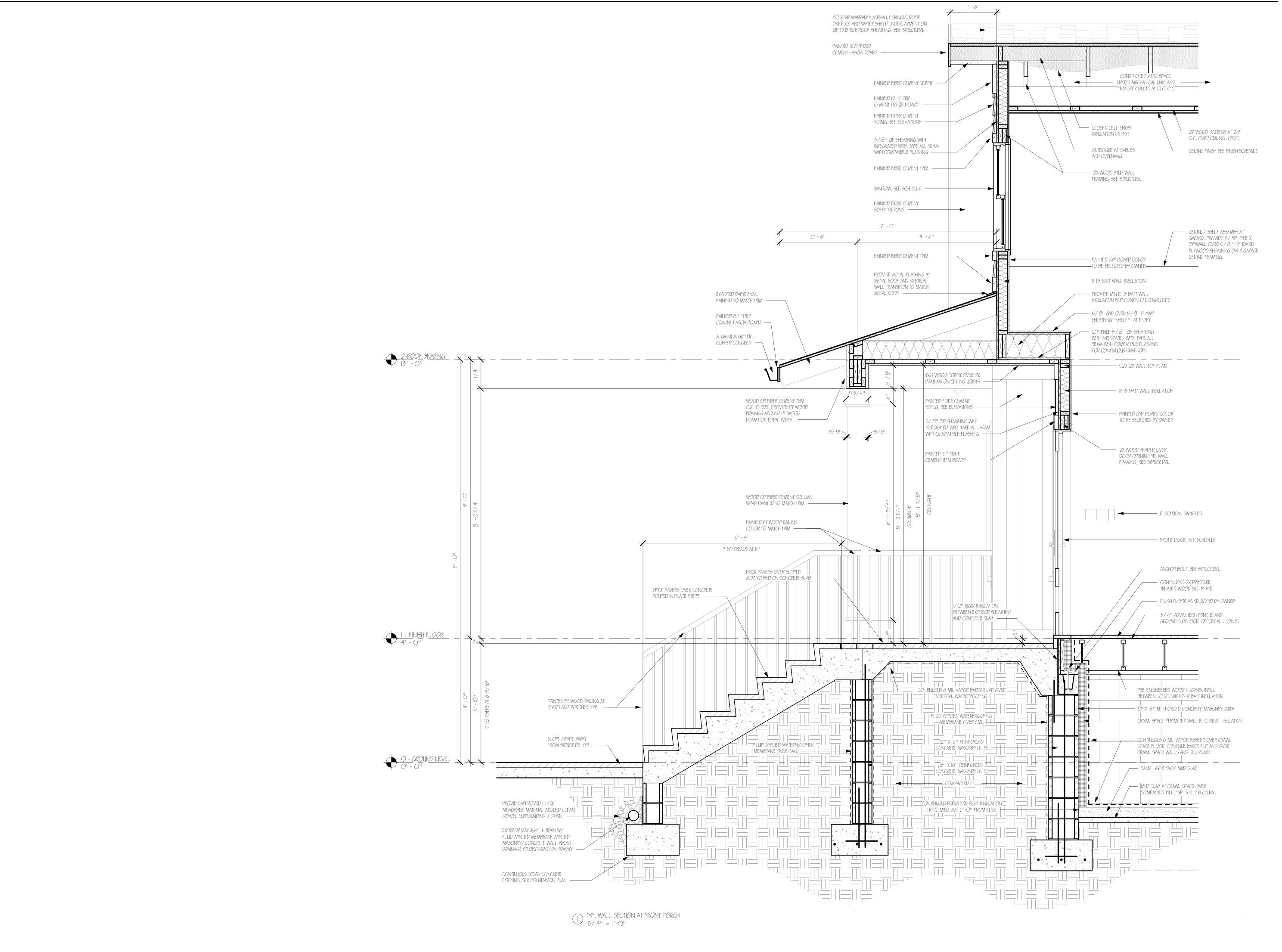
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07/31/2023





HARRISSMITH architects

6278 N FEDERAL HWY #118 FORT LAUDERDALE, FLORIDA 33308

TEL: 757.739.5200

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NO. DESCRIPTION DATE

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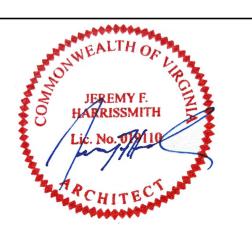
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OWNER:
TERRY INDUSTRIES

2509 GEORGE MASON DR., #6894 VIRGINIA BEACH, VIRGINIA 23456

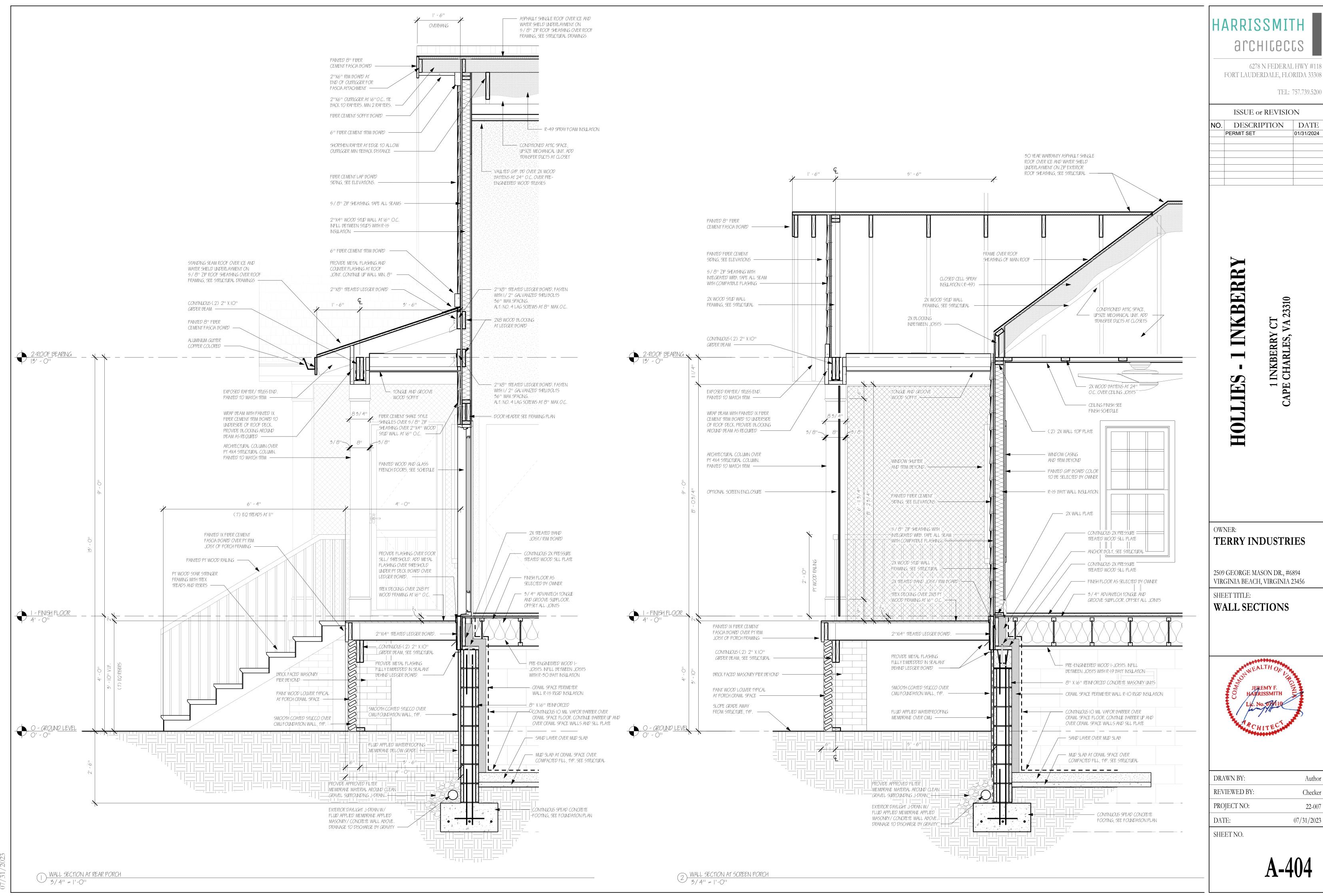
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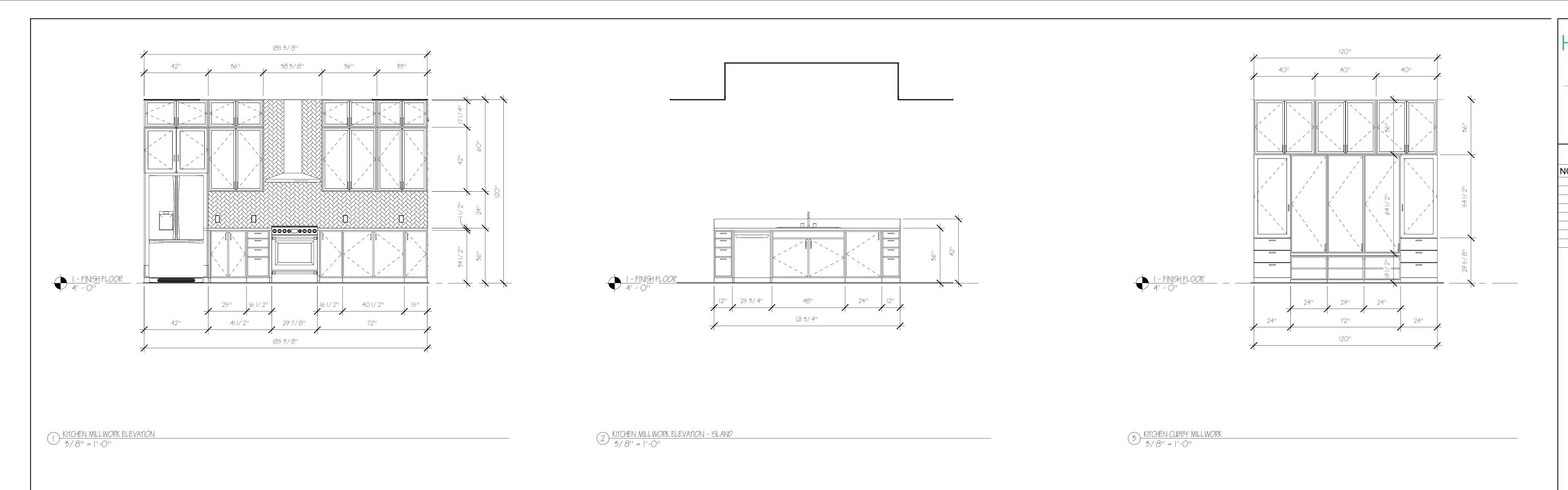
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architects

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OWNER: TERRY INDUSTRIES

2509 GEORGE MASON DR., #6894 VIRGINIA BEACH, VIRGINIA 23456

SHEET TITLE:

MILLWORK ELEVATIONS -KITCHEN



DRAWN BY:	JFH
REVIEWED BY:	JFH
PROJECT NO:	22-007
DATE:	07/31/2023
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## GENERAL NOTES

- USE STRUCTURAL DRAWINGS IN CONJUNCTION WITH ALL OTHER DRAWINGS COORDINATE THE WORK OF OTHER TRADES INCLUDING BUT NOT LIMITED TO THE REQUIREMENTS FOR SLEEVES, INSERTS, HOLES, HANGERS AND ANCHORS.
- 2. ELEVATIONS ON THE STRUCTURAL DRAWINGS ARE DENOTED AS [+/- X'-X"] REFERENCED TO THE FINISHED FIRST FLOOR ELEVATION DATUM. REFER TO THE CIVIL DRAWINGS FOR ACTUAL DATUM ELEVATION.
- 3. REPORT DISCREPANCIES IN DIMENSIONS BETWEEN DIFFERENT DRAWINGS TO THE ARCHITECT PRIOR TO BEGINNING WORK IN AREAS THAT WILL BE AFFECTED.
- 4. DETAILS ENTITLED AS "TYPICAL" APPLY NOT ONLY WHERE SPECIFICALLY INDICATED OR REFERENCED, BUT ALSO IN ALL OTHER CASES WHERE THE NATURE OF THE CONSTRUCTION REQUIRES THEIR USE. DETERMINE APPLICABILITY OF TYPICAL DETAILS FROM DESCRIPTIVE TITLES OR FROM SIMILARITY OF A CONSTRUCTION CONDITION WHERE THE DETAIL IS SPECIFICALLY INDICATED OR REFERENCED
- 5. THE STRUCTURAL DRAWINGS CONTAINED HEREIN REPRESENT THE FINISHED STRUCTURE. PROVIDE ALL TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL ALL STRUCTURAL WORK, INCLUDING CONNECTIONS IS COMPLETE. ANALYSIS, DESIGN, SAFETY, ADEQUACY AND INSPECTION OF ERECTION BRACING, SHORING, AND OTHER TEMPORARY SUPPORTS ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 6. CONSTRUCTION MEANS, METHODS, TECHNIQUES, AND SEQUENCES AND SUPERVISION OF THE WORK ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR
- 7. REPRODUCTION OF CONTRACT DRAWINGS SHALL NOT BE USED AS SHOP DRAWINGS UNDER ANY CIRCUMSTANCE
- 8. SUBMIT FINAL WOOD TRUSS SHOP DRAWINGS, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE COMMONWEALTH OF VIRGINIA.TO THE STRUCTURAL ENGINEER PRIOR TO COMMENCEMENT OF WORK TO CONFIRM ADEQUACY OF FOUNDATIONS AND SHEARWALLS.

## DESIGN NOTES

- I. STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE FOLLOWING CODES AND SPECIFICATIONS:
- A. 2018 INTERNATIONAL RESIDENTIAL CODE
- B. 2018 VIRGINIA RESIDENTIAL CODE
- C. ASCE/SEI 7-16, MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES
- ACI 318-14, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
- TMS 402-16, BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES F. ANSI/AWC NDS -2018 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION WITH 2018 SUPPLEMENT
- 2. DESIGN LOAD CRITERIA
- A. LIVE LOADS (UNIFORM)

FLOOR (DWELLING) GARAGE 50 PSF

NOTE: LIVE LOAD REDUCTION WAS NOT USED IN THE DESIGN OF THIS STRUCTURE

B. SNOW LOADS:

GROUND SNOW LOAD, P 10 PSF SNOW EXPOSURE FACTOR. Ce 1.0 THERMAL FACTOR, Ct 1.0 SNOW LOAD IMPORTANCE FACTOR. IS 1.0

C. WIND LOADS: BASIC WIND SPEED, VIII T

120 MPH 93 MPH RISK CATEGORY WIND EXPOSURE CATEGORY GUST EFFECT FACTOR, G 0.85 INTERNAL PRESSURE COEFFICIENTS, GCDI +/-0.18

D. SEISMIC LOADS:

SITE CLASS (ASSUMED) SEISMIC DESIGN CATEGORY

## FOUNDATION NOTES

- SELECT FILL MATERIAL SHALL BE CLASSIFIED AS GW GP, SW OR SP BY ASTM D2487. THE LIQUID LIMIT OF SUCH MATERIAL SHALL NOT EXCEED 35 PERCENT WHEN TESTED IN ACCORDANCE WITH ASTM D4318. THE PLASTICITY INDEX SHALL NOT BE GREATER THAN 12 PERCENT WHEN TESTED IN ACCORDANCE WITH ASTM D4318, AND NOT MORE THAN 35 PERCENT BY WEIGHT SHALL BE FINER THAN 75 MICROMETERS No. 200 SIEVE WHEN TESTED IN ACCORDANCE WITH ASTM DI140.
- 2. DESIGN ALLOWABLE SOIL BEARING PRESSURE IS 1,500 PSF ON SUITABLE RESIDUAL SOIL OR PROPERLY COMPACTED SELECT FILL. THE ALLOWABLE SOIL BEARING PRESSURE IS AN ASSUMED VALUE. SELECT FILL SHALL BE COMPACTED TO A MINIMUM OF 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY (ASTM D-698).
- 3. PRIOR TO PLACING FOUNDATION CONCRETE, THE CONTRACTOR SHALL ENSURE THAT THE FOUNDATION EXCAVATIONS ARE INSPECTED BY AN INDEPENDENT TESTING LABORATORY AND GEOTECHNICAL ENGINEER REGISTERED IN THE COMMONWEALTH OF VIRGINIA TO EVALUATE THE EXTENT OF LOOSE, SOFT OR OTHERWISE UNSATISFACTORY SOIL MATERIAL AND TO VERIFY THE DESIGN BEARING CAPACITY. SOILS NOT SUITABLE FOR FOUNDATION SUPPORT SHALL BE UNDERCUT AND REPLACED WITH SELECT FILL
- 4. ADEQUATELY PROTECT FOUNDATION EXCAVATIONS TO PREVENT WATER FROM ACCUMULATING AND STANDING IN THE EXCAVATION BOTTOMS
- 5. DO NOT PLACE FOUNDATION CONCRETE ON FROZEN OR SATURATED SUBGRADES.
- 6. ENSURE THAT EARTH-FORMED FOOTINGS CONFORM TO THE SHAPE, LINES AND THICKNESSES INDICATED ON THE FOUNDATION PLAN. EXCAVATION WIDTHS SHALL BE A MINIMUM OF 4 INCHES GREATER THAN DIMENSIONS INDICATED
- 7 PLACE FOUNDATION CONCRETE THE SAME DAY EXCAVATIONS ARE MADE OR AS SOON AS PRACTICAL THEREAFTER
- 8. DO NOT INSTALL FOUNDATIONS UNTIL FOUNDATION WORK HAS BEEN COORDINATED WITH ADJACENT UNDERGROUND UTILITIES AND STRUCTURES.
- 9. FOOTINGS SHALL BE LOWER AS REQUIRED TO PASS UNDER UTILITY LINES. STEP CONTINUOUS FOOTINGS DOWN AS SHOWN IN THE "TYPICAL STEPPED FOOTING" DETAIL
- 10. POROUS FILL SHALL BE CLEAN CRUSHED ROCK CONFORMING TO COARSE AGGREGATE SIZE 57 OR NATURAL SAND.

- I. ALL MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (TMS 402-2016) AND "SPECIFICATION FOR MASONRY STRUCTURES" (TMS 602 -2016).
- 2. DESIGN MASONRY ASSEMBLAGE STRENGTH, F'm = 2000 PSI. NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS SHALL BE A MINIMUM OF 2000 PSI.
- 3. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND BE MANUFACTURED WITH LIGHTWEIGHT AGGREGATE
- 4. GROUT SHALL CONFORM TO ASTM C476 AND SHALL NOT CONTAIN ADMIXTURES. GROUT SHALL ATTAIN A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2000 PSI.
- 5. GROUT POURS SHALL BE STOPPED 1-1/2" INCHES BELOW THE TOP OF A COURSE TO FORM A KEY AT POURED JOINTS.
- 6. REINFORCEMENT SHALL BE DEFORMED BARS CONFORMING TO ASTM A615/ A615M. GRADE 60 AND SHALL HAVE FABRICATION TOLERANCES IN ACCORDANCE WITH ACI 315. SHOP FABRICATE REINFORCING BARS WHICH ARE INDICATED TO BE BENT OR HOOKED.
- 7. LOCATE JOINT REINFORCEMENT 16 INCHES ON CENTER VERTICALLY, PROVIDE ADDITIONAL REINFORCEMENT AT TOP OF ALL FOUNDATIONS AND IN THE TWO JOINTS IMMEDIATELY ABOVE AND BELOW ALL OPENINGS. EXTEND JOINT REINFORCEMENT A MINIMUM OF 24 INCHES BEYOND THE OPENING OF EACH
- 8. PLACE PIPES AND CONDUITS PASSING HORIZONTALLY THROUGH MASONRY IN STEEL OR PYC SLEEVES OR CORED HOLES UNLESS OTHERWISE INDICATED ON THE DRAWINGS.
- 9. ALUMINUM CONDUITS, PIPES, AND ACCESSORIES SHALL NOT BE EMBEDDED IN MASONRY GROUT, OR MORTAR, UNLESS EFFECTIVELY COATED OR COVERED TO PREVENT ALUMINUM-CEMENT CHEMICAL REACTION AND ELECTROLYTIC ACTION BETWEEN ALUMINUM AND STEEL
- 10. UNLESS OTHERWISE NOTED OR DETAILED, CENTER WALL REINFORCEMENT IN BLOCK CELLS. USE NONMETALLIC BAR POSITIONERS
- II. PROVIDE DOWEL REINFORCEMENT FROM FOUNDATION OF SAME SIZE AND SPACING AS VERTICAL WALL REINFORCEMENT. LAP WALL REINFORCEMENT A MINIMUM OF 72 BAR DIAMETERS UNLESS OTHERWISE NOTED.
- 12. DO NOT USE FROZEN MATERIALS OR MATERIALS MIXED OR COATED WITH ICE OR FROST. DO NOT BUILD ON FROZEN SUBSTRATES. REMOVE AND REPLACE UNIT MASONRY DAMAGED BY FROST OR BY FREEZING CONDITIONS.
- 13. MORTAR SHALL BE TYPE M OR S PREPARED IN ACCORDANCE WITH ASTM C270.

## CONCRETE NOTES

- ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF THE AMERICAN CONCRETE INSTITUTE (ACI) SPECIFICATIONS FOR STRUCTURAL CONCRETE (ACI-301-16). ALL CONCRETE DESIGN PERFORMED BY THE CONTRACTOR SHALL BE IN ACCORDANCE WITH ACI 318-14.
- 2. CAST-IN-PLACE CONCRETE SHALL ATTAIN THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTH (f'c):

A. SLABS-ON-GRADE

3000 PSI 3000 PSI

- 3. CONCRETE DENSITY SHALL BE NORMAL WEIGHT UNLESS SPECIFICALLY OTHERWISE NOTED
- 4. CONCRETE REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615/ A615M, GRADE 60.
- 5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064. PROVIDE SHEET-TYPE WELDED WIRE FABRIC. SHEET LAPS SHALL BE TIED AND LAPPED ONE FULL MESH SPACING PLUS 2".
- 6. CONCRETE REINFORCING STEEL SHALL BE CONTINUOUS UNLESS OTHERWISE INDICATED. CONTINUOUS STEEL SHALL BE LAPPED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318 (MINIMUM 25").
- 7. MINIMUM CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS INDICATED IN NO CASE SHALL REINFORCEMENT COVER LESS THAN THE REQUIREMENTS OF
  - A. CONCRETE DEPOSITED AGAINST THE GROUND B. CONCRETE EXPOSED TO EARTH OR WEATHER
- 8. CONCRETE REINFORCING STEEL MARKED STANDARD HOOK SHALL HAVE A 90-DEGREE HOOK UNLESS OTHERWISE NOTED. STIRRUPS, TIES, 180-DEGREE HOOKS AND 90-DEGREE HOOKS SHALL CONFORM TO THE REQUIREMENTS OF ACI 318.
- 9. PROVIDE 1/2" THICK JOINT FILLER MATERIAL WHERE SLABS ON GRADE ABUT VERTICAL SURFACES
- IO. REINFORCING STEEL SHALL BE SPREAD AT SLEEVES, TIEBACKS, RECESSES AND OTHER EMBEDDED ITEMS UNLESS OTHERWISE INDICATED. REINFORCEMENT SHALL NOT BE CUT TO FACILITATE PLACEMENT OF EMBEDDED ITEMS.
- II. NO CONCRETE SHALL BE PLACED UNTIL THE OWNER OR OWNER'S REPRESENTATIVE HAS INSPECTED ALL EMBEDDED WORK, INCLUDING REINFORCEMENT.
- 12. ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED 3/4" OR AS INDICATED.
- 13. ALUMINUM SHALL NOT BE PLACED IN DIRECT CONTACT WITH CONCRETE UNLESS EFFECTIVELY COATED OR COVERED TO PREVENT ALUMINUM-CONCRETE REACTION AND ELECTROLYTIC ACTION BETWEEN ALUMINUM AND STEEL.
- 14. ALL EXPOSED CONCRETE WALL SURFACES SHALL RECEIVE A SMOOTH RUBBED FINISH.
- 15. FIBER REINFORCING SHALL BE POLYPROPYLENE/ POLYETHYLENE MACRO FIBER AND CONFORM TO ASTM D7508 ANS ASTM C116 TYPE 111.
- 16. FIBER REINFORCED CONCRETE SHALL PROVIDE A MINIMUM Re3 VALUE OF 19.2% WHEN TESTED IN ACCORDANCE WITH ASTM C1609

#### STEEL NOTES

- FABRICATION AND ERECTION OF STRUCTURAL STEEL AND DESIGN OF CONNECTIONS SHALL BE IN ACCORDANCE WITH THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" DATED JULY 7, 2016 AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" DATED JUNE 15, 2016.
- 2. UNLESS OTHERWISE NOTED, STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE ABOVE-LISTED AISC SPECIFICATION AND THE FOLLOWING:
- A. PLATES AND ANGLES

B. HIGH STRENGTH BOLTS

ASTM A36/ A36M ASTM A325

Fb=2900 PSI

Fy=285 PSI

C. ANCHOR RODS W/ NUT AND WASHER

ASTM FI554, GRADE 36

3. ALL STEEL, INCLUDING NAILS, EXPOSED TO PRESSURE-TREATED LUMBER SHALL BE HOT-DIP GALVANIZED PER ASTM A123.

# ENGINEERED WOOD PRODUCT NOTES

I. ALL ENGINEERED WOOD PRODUCTS (EWP) SHALL HAVE THE FOLLOWING MINIMUM DESIGN PROPERTIES:

A. LVL (I 3/4")

E=2x106 PS1 Fc=750 PSI PERP.

B. POST

E=1.9x106 PS1 Fb=2100 PSI Fc=2300 PSI Fy=1825 PSI

INTERNATIONAL BEAMS 18400 OR EQUAL C. FLOOR JOIST

# STRUCTURAL LEGEND

ANCHOR BOLT

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#### SYMBOL DESCRIPTION

AFF ABOVE FINISHED FLOOR AHR ANCHOR APPROX APPROXIMATELY ARCH ARCHITECTURAL BD BAR DIAMETER BOC BOTTOM OF CONCRETE BOM BOTTOM OF MASONRY BOTTOM OF STEEL BRG BEARING BML BRACED WALL LINE BRACED WALL PANEL BMP CFS COLD-FORMED STEEL SLAB CONSTRUCTION JOINT CENTERLINE CLR CLEAR CONCRETE MASONRY UNIT COL COLUMN CONN CONNECTION

CONC CONCRETE CONT CONTINUOUS LOD DOMELED CONSTRUCTION JOINT DIAMETER DIA. P

DOWN DRAWING(S. DWG(S) EACH EACH FACE ELEVATION EDGE OF SLAB EQ EQUAL ΕM EACH WAY

FINISHED FLOOR ELEVATION FOB FACE OF BRICK FTG FOOTING GAGE GALY GALVANIZED HOLD DOWN

HORIZONTAL

HIGH STRENGTH

JBE JOIST BEARING ELEVATION KIP (K) 1000 POUNDS LONG LEG HORIZONTAL LONG LEG VERTICAL LONG SIDE HORIZONTAL

LSV LONG SIDE VERTICAL MAXIMUM MANUFACTURER MIN MINIMUM MIDDLE OF WALL

> NOT IN CONTRACT NUMBER No. NOT TO SCALE 00 ON CENTER OPPOSITE

PREMOLDED JOINT FILLER POUNDS PER LINEAR FOOT PSF POUNDS PER SQUARE FOOT

HORIZ

REINFORCEMENT REINF REQD REQUIRED SCHED SCHEDULE SLAB DEPRESSION

SF STEPPED FOOTING SIM SIMILAR SJ SLAB SAWED (CONTRACTION) JOINT SLOPE(D)

506 SLAB-ON-GRADE STD STANDARD SM SHEARWALL T&B TOP AND BOTTOM TOP OF CONCRETE

TOF TOP OF FOOTING TOM TOP OF MASONRY TOS TOP OF STEEL TOSL TOP OF SLAB TS THICKENED SLAB UON UNLESS OTHERWISE NOTED

VERT VERTICAL WORKING POINT MMF WELDED WIRE FABRIC

SLOPE DIRECTION DNDEPRESSED SLAB ////// DECK SPAN

INDICATES ELEVATION (±X'-X") REFERENCED TO FINISH FLOOR

 $M \times$ / **)**\_\_\_\_\_

MALL TYPE COLUMN REFERENCE LINE (CENTERLINE OF COLUMN)

KEYED CONSTRUCTION NOTE

[±X'-X"]

SPOT ELEVATION

BRICK CONCRETE

GROUT

POROUS FILL LOAD BEARING

STUD WALL

CONCRETE MASONRY UNIT

SIONAL ET 2876 PROJ NO. DATE: 8/14/2023 EARTH FILL NONE SCALE: CHECKED: GSF LRC

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NEALTHO

LOUIS RHETT CRIBB

Lic. No. 0402065775

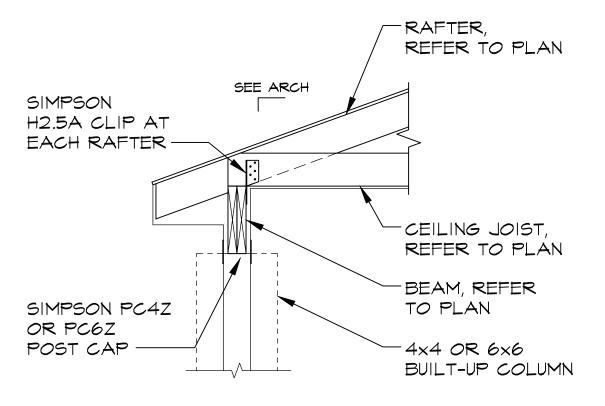
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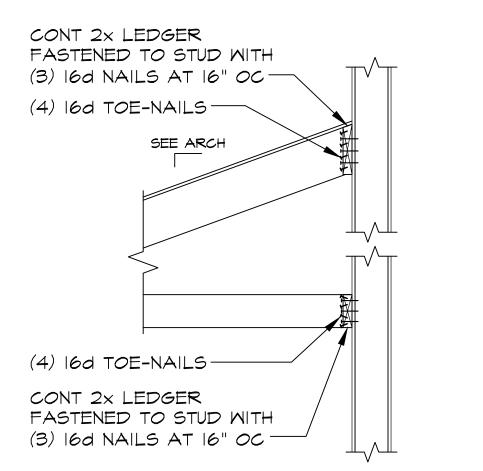
# ROUGH CARPENTRY NOTES

- DESIGN OF ALL ROUGH CARPENTRY CONSTRUCTION IS ACCORDANCE WITH 2018 EDITION OF THE AMERICAN WOOD COUNCIL 'NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION', INCLUDING SUPPLEMENTS.
- 2. ALL DIMENSION LUMBER FRAMING MEMBERS SHALL COMPLY WITH PS-20 'AMERICAN SOFTWOOD LUMBER STANDARD'. MAXIMUM MOISTURE CONTENT SHALL BE 19 PERCENT. SPECIES AND GRADE SHALL BE AS FOLLOWS:
  - A. LOAD BEARING WALL FRAMING SOUTHERN PINE NO. 2 OR BETTER B. JOISTS, RAFTERS, BEAMS, HEADERS CHORD MEMBER FRAMING-SOUTHERN PINE NO.2 OR AS INDICATED
  - C. SPRUCE PINE FUR IS NOT PERMITTED FOR STRUCTURAL WOOD FRAMING
- 3. WOOD STRUCTURAL PANELS SHALL BE PLYWOOD COMPLYING WITH PS-I 'STRUCTURAL PLYWOOD' OR ORIENTED STRAND BOARD IN ACCORDANCE WITH PS-2, 'PERFORMANCE STANDARD FOR WOOD-BASED STRUCTURAL-USE PANELS' AND THE FOLLOWING REQUIREMENTS:
  - A. EXTERIOR WALL SHEATHING: 7/16" APA SHEATHING EXPOSURE
- B. SHEARWALL SHEATHING: APA RATED SHEATHING, EXPOSURE | AS INDICATED
- C. FLOOR SHEATHING: 3/4" APA RATED SHEATHING, EXPOSURE
- D. ROOF SHEATHING: 7/16" APA RATED SHEATHING, EXPOSURE
- 4. SPLICES OF TOP PLATES IN EXTERIOR WALLS SHALL CONSIST OF (12) 16d COMMON NAILS, EACH SIDE OF SPLICE.
- 5. FASTEN EXTERIOR WALL SHEATHING TO FRAMING WITH 8d NAILS SPACED AT 6" OC AT PANEL EDGES AND 12" OC AT INTERMEDIATE SUPPORTS, UNLESS OTHERWISE NOTED. ALL EDGES SHALL BE SUPPORTED WITH BLOCKING.
- 6. INSTALL/ALIGN LOAD BEARING WALL STUDS DIRECTLY UNDER ROOF GIRDERS, FLOOR GIRDERS, BEAMS AND HEADERS. INSTALL ONE STUD FOR EACH PLY OF GIRDER, BEAM OR HEADER,
- 7. INSTALL ROOF AND FLOOR SHEATHING WITH LONG DIMENSIONS PERPENDICULAR TO SUPPORTS. SHEATHING MUST BE CONTINUOUS OVER TWO OR MORE SPANS. STAGGER JOINTS IN ADJACENT PANELS. ATTACH PANELS WITH IOD NAILS AT 6" OC AT SUPPORTED EDGES AND AT 12" OC AT INTERMEDIATE SUPPORTS, REFER TO "TYPICAL ROOF DIAPHRAGM" DETAIL FOR OTHER NAILING AND BLOCKING REQUIREMENTS.
- 8. UNLESS OTHERWISE NOTED, ALL NAILING SHALL COMPLY WITH TABLES R602.3(1)AND R602.3(3) IN THE INTERNATIONAL RESIDENTIAL CODE.
- 9. SILL PLATES SHALL BE PRESSURE-TREATED
- IO. UNLESS OTHERWISE NOTED, ATTACH BLOCKING AND NAILERS TO STEEL FRAMING USING 3/16" DIAMETER POWDER ACTUATED FASTENERS AT 24" OC OR 1/2" DIAMETER ANCHOR BOLTS AT 24" OC
- II. BOLTS CONNECTING WOOD MEMBERS SHALL BE FABRICATED IN ACCORDANCE WITH ASTM A307.
- 12. UNLESS OTHERWISE NOTED OR DETAILED, CONNECT ALL SILL PLATES TO FLOOR SLAB WITH 1/2" DIAMETER ANCHOR BOLTS WITH 7" EMBEDMENT
- 13. ANCHOR CAPACITIES AND SHEAR FORCES ARE ALLOWABLE STRESS DESIGN



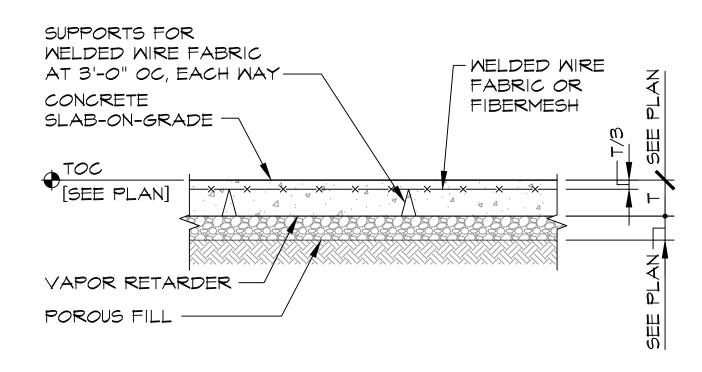
# TYPICAL PORCH BEARING AT BEAM

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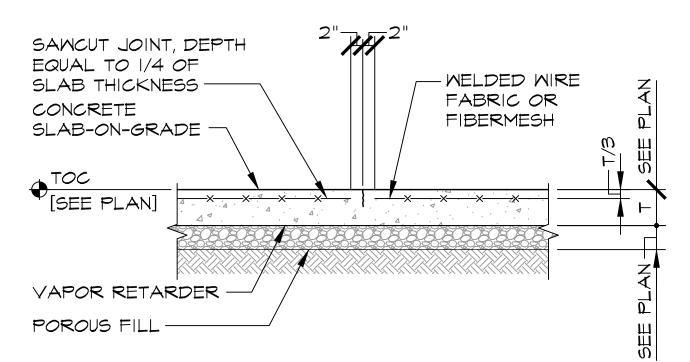


# TYPICAL PORCH BEARING AT WALL

NOT TO SCALE



# TYPICAL SLAB-ON-GRADE (SOG) NOT TO SCALE



# TYPICAL SLAB-ON-GRADE SAMED (CONTRACTION) JOINT (SJ) NOT TO SCALE

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Lic. No. 0402065775

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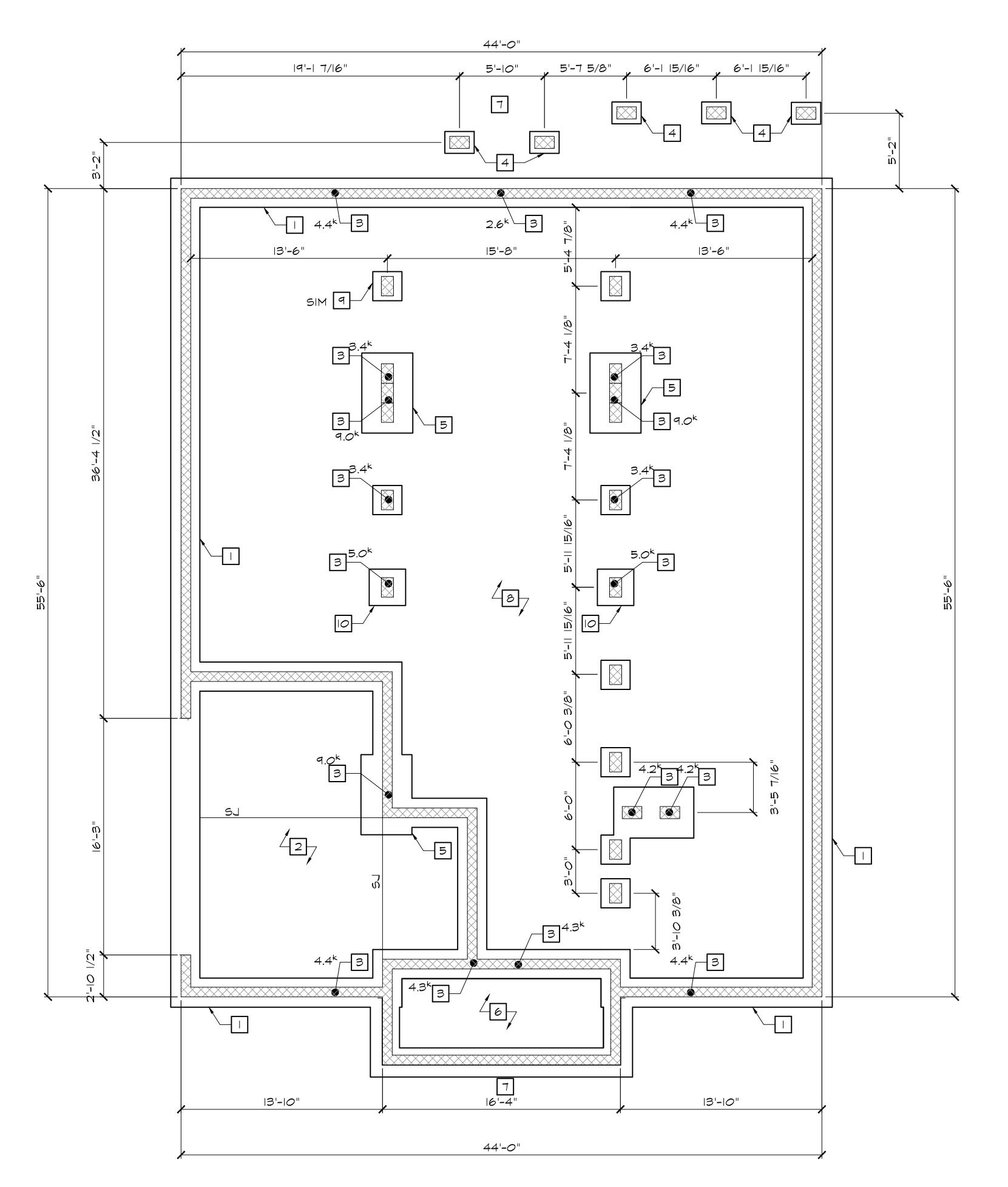
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SHEET 2 OF 10

- I. TOP OF SLAB-ON-GRADE ELEVATION IS REFERENCE ELEVATION [0'-0"].
- 2. REFER TO TYPICAL FOUNDATION SECTIONS FOR TOP OF FOOTING ELEVATION.
- 3. SLAB-ON-GRADE IS 5" THICK, REINFORCED WITH FIBERMESH.
  PLACE SLAB-ON-GRADE ON 6 MIL VAPOR RETARDER OVER
  4" POROUS FILL.
- 4. REFER TO TYPICAL DETAIL FOR SLAB CONTRACTION JOINT (SJ) LOCATIONS INDICATED ON PLAN.
- 5. REFER TO TYPICAL DETAIL FOR MUD SLAB LOCATIONS INDICATED ON PLAN.
- 6. REFER TO ARCHITECTURAL PLANS FOR LAYOUT OF INTERIOR PARTITION WALLS AND DIMENSIONS NOT NOTED.
- 7. WALL CONSTRUCTION, UNLESS OTHERWISE NOTED, SHALL BE 2×4 STUDS AT 16" OC WITH DOUBLE 2×4 TOP PLATES AND A SINGLE 2×4 BOTTOM PLATE. REFER TO GANG STUD COLUMN SCHEDULE FOR ADDITIONAL INFORMATION.
- 8. WALLS SHOWN ON STRUCTURAL PLANS ARE LOAD BEARING WALLS.
- 9. CENTER FOOTINGS UNDER COLUMNS AND WALLS.
- IO. CONCENTRATED LOADS ARE INDICATED AS  $X^K$  AND ARE ALLOWABLE STRESS DESIGN (ASD) VALUES

# KEY NOTES

- CONTINUOUS 2'-O" X 1'-O" THICK FOOTING REINFORCED WITH (3) #4 BOTTOM, REFER TO TYPICAL SECTIONS ON SB501
- 2 5" THICK CONCRETE SLAB-ON-GRADE
- 3 REFER TO GANG STUD COLUMN SCHEDULE
- 4 2'-0"  $\times$  1'-6"  $\times$  1'-0" DEEP FOOTING REINFORCED WITH (3) #4 TOP AND BOTTOM, EACH WAY
- $5'-6" \times 3'-6" \times 1'-0"$  DEEP FOOTING REINFORCED WITH (8) #4 TOP AND BOTTOM, EACH WAY
- 6 PORCH FOUNDATION, REFER TO 'TYPICAL SECTION AT PORCH DETAIL.
- 7 CONCRETE STEPS, REFER TO ARCH DWGS
- 8 2" THICK CONCRETE MUD SLAB
- 9 2'-0" × 2'-0" × 1'-0" DEEP FOOTING REINFORCED WITH (3) #4 TOP AND BOTTOM, EACH WAY
- 0 2'-6"  $\times$  2'-6"  $\times$  1'-0" DEEP FOOTING REINFORCED WITH (4) #4 TOP AND BOTTOM, EACH WAY



FOUNDATION PLAN

SCALE: 3/16" = 1'-0"

Sineering Services
LE DESIGN, LLC
V. WASHINGTON STREET, SUITE 2
OLK, VIRGINIA 23434
VE:757.809.3700

SIMPLE 521 W. SUFFOI PHONE WWW.S

INKBERRY

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LOUIS RHETT CRIBB
Lic. No. 0402065775

8/14/2023

PROJ NO. 2876

DATE: 8/14/2023

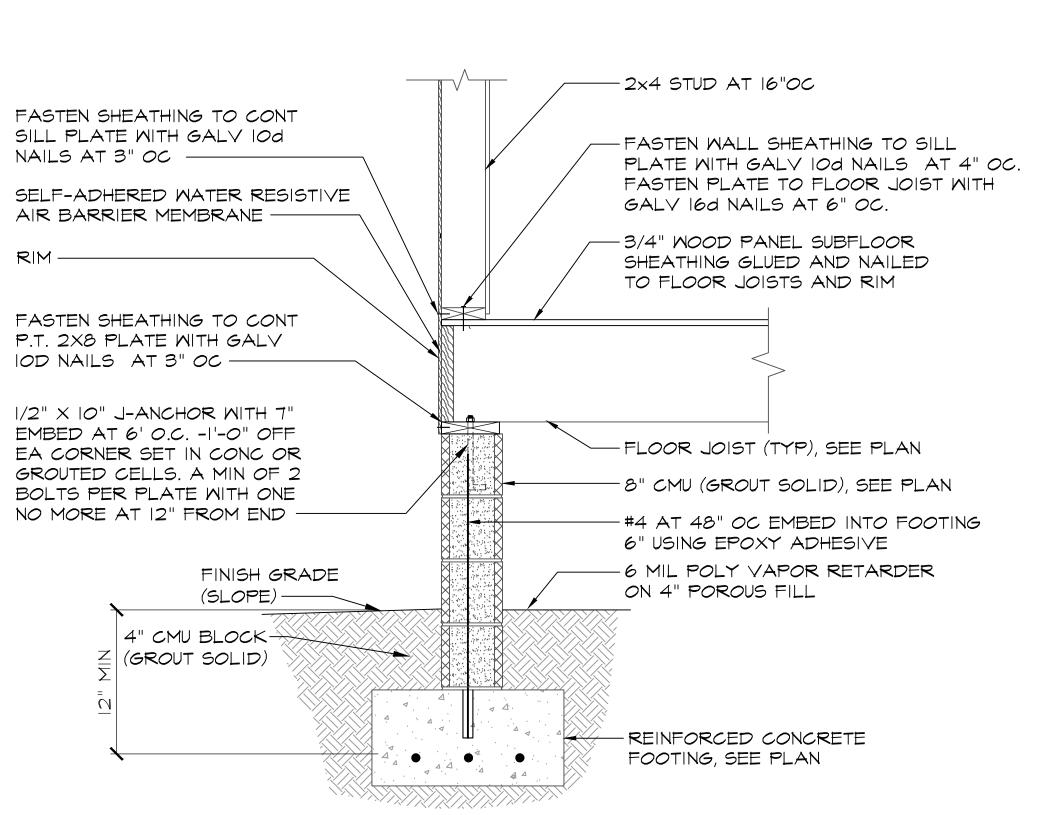
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SHEET 3 OF 10



TYPICAL SECTION THROUGH EXTERIOR WALL

NOT TO SCALE

2×4 / 2×6 MALL AT 16" OC (REFER TO PLANS) -1/2"x|8" AB AT 6'-0" OC, -12" OFF EACH CORNER SET IN CONCRETE OR GROUTED CELLS. A MINIMUM OF 2 BOLTS PER PLATE WITH ONE NOT MORE THAN 12" FROM END -2×8 TREATED PLATE #4 AT 48"o.c.— -FLASHING AS REQD 8"x16" BLOCK -CONCRETE SLAB-ON-GRADE, CMU BLOCKS REFER TO PLAN (GROUT SOLID)-FINISH GRADE (SLOPE) -VAPOR RETARDER - POROUS FILL CONT 24" WIDE x 12" THICK CONCRETE FOOTING #4 AT 16"0.c.

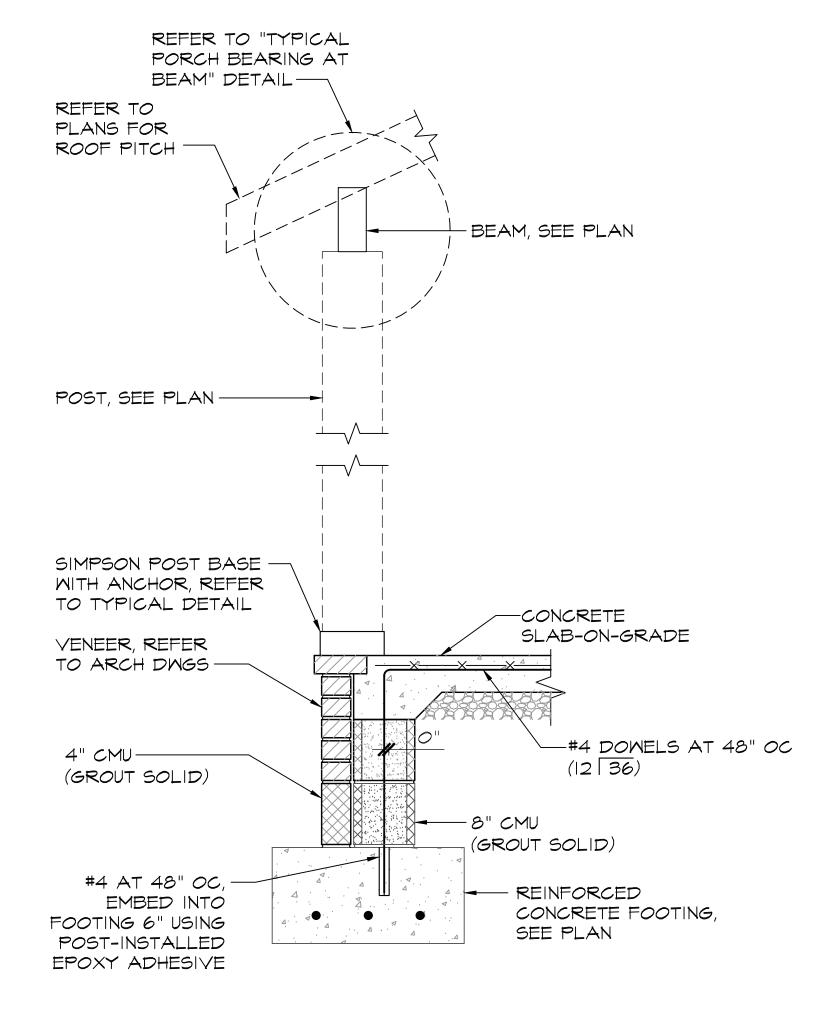
-1/2"x18" AB AT 6'-0" OC, -12" OFF EACH CORNER SET IN CONCRETE OR GROUTED CELLS. A MINIMUM OF 2 BOLTS PER PLATE WITH ONE NOT MORE THAN 12" FROM END 2"x24" PERIMETER INSULATION #4 AT 48"o.c.-- FLOOR JOIST, SEE PLAN CONCRETE SLAB-ON-GRADE. REFER TO PLAN (SLOPE) - I" PERIMETER INSULATION -8"x16"CMU SHOE BLOCK -8"x16" CMU BLOCK POROUS FILL (TYP)-(GROUT SOLID) VAPOR RETARDER (TYP)--CONT 24" WIDE  $\times$  12" THICK CONCRETE FOOTING #4 AT 16"o.c.—

TYPICAL EXTERIOR AT EXTERIOR WALL SECTION AT GARAGE WALL

INTERIOR GARAGE WALL SECTION NOT TO SCALE

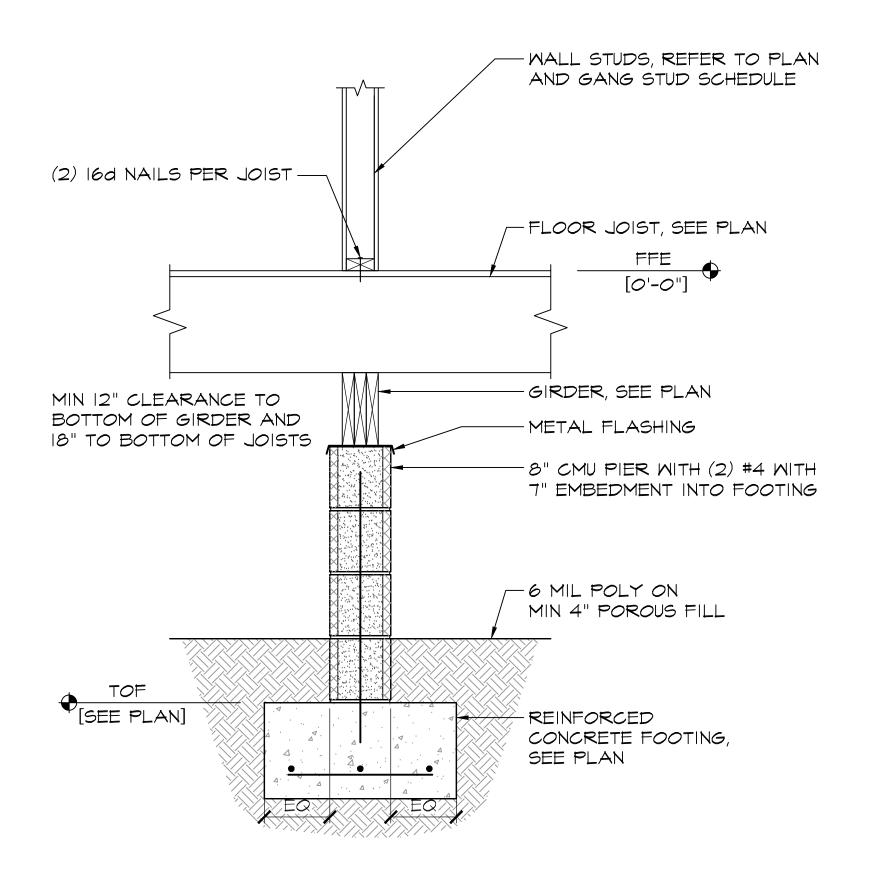
REFER TO PLANS FOR

WALL FINISH INFORMATION

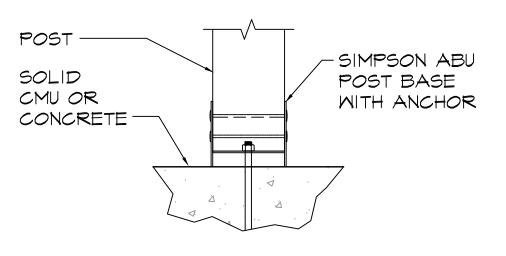


NOT TO SCALE

TYPICAL SECTION AT PORCH NOT TO SCALE







POST HOLD DOWN DETAIL
NOT TO SCALE

PROJ NO. 2876
DATE: 8/14/2023
SCALE: AS SHOWN
CHECKED: GSF
DRAWN BY: LRC

LOUIS RHETT CRIBB

Lic. No. 0402065775

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INKBERRY

DATE

23310

CHARLES, VA

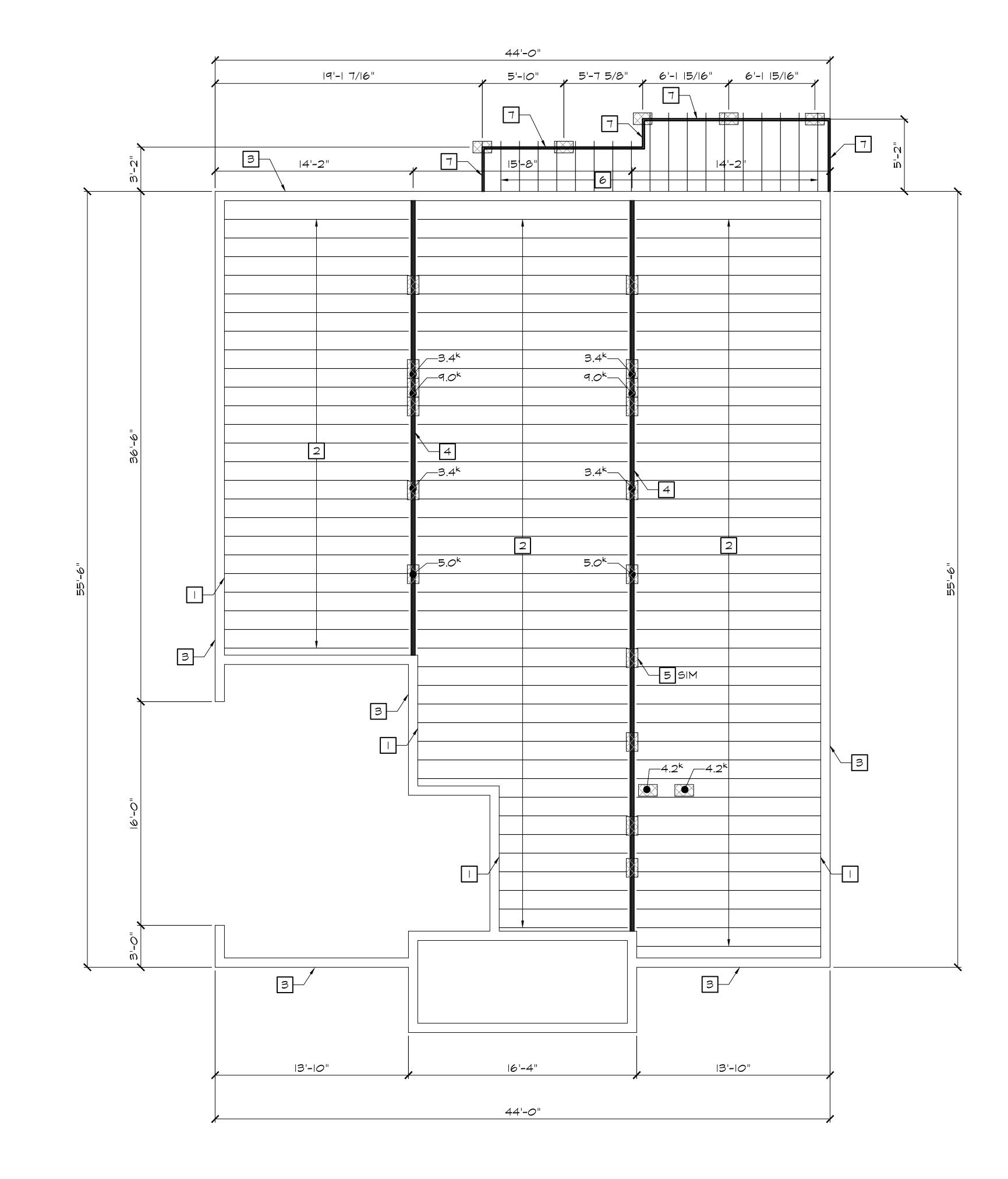
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SHEET 4 OF 10

- I. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT NOTED.
- 2. FLOOR CONSTRUCTION, UNLESS OTHERWISE NOTED, SHALL BE 12"
  PREFABRICATED FLOOR JOISTS OR METAL-PLATE-CONNECTED
  WOOD FLOOR TRUSSES WITH WOOD PANEL SHEATHING ON ALL
  TOP CHORD SURFACES. REFER TO 'ROUGH CARPENTRY NOTES'
  FOR WOOD PANEL SPECIFICATIONS.
- 3. METAL-PLATE-CONNECTED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE INTERNATIONAL RESIDENTIAL CODE AND THE TPI I-2014, 'NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION.'
- 4. HANDLING, INSTALLING, RESTRAINING AND BRACING OF METAL-PLATE-CONNECTED WOOD TRUSSES SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE BCSI, 'BUILDING COMPONENT SAFETY INFORMATION' GUIDE.
- 5. DESIGN METAL-PLATE-CONNECTED WOOD FLOOR TRUSSES FOR THE FOLLOWING SUPERIMPOSED LOADS IN ADDITION TO THE LOADS INDICATED IN 'DESIGN NOTES' ON SHEET S-001 OR OTHERWISE NOTED:
- A. TOP CHORD DEAD LOAD
- 10 PSF
- B. BOTTOM CHORD DEAD LOAD
- 6. INCLUDE SECONDARY BENDING STRESSES DUE TO SUPERIMPOSED LOADS IN THE DESIGN OF CHORD MEMBERS.
- 7. LIMIT MID-SPAN DEFLECTION OF THE TOP OF EACH FLOOR
  JOIST OR TRUSS DUE TO LIVE LOAD TO SPAN/360. LIMIT
  MID-SPAN DEFLECTION OF THE TOP OF EACH FLOOR JOIST OR
  TRUSS DUE TO TOTAL LOAD TO SPAN/240.
- 8. ALL METAL-PLATE-CONNECTORS AND OTHER FASTENERS SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM AI23 WHEN IN CONTACT WITH PRESSURE TREATED AND FIRE RETARDANT LUMBER AND WHEN TRUSSES ARE INSTALLED WITHIN ONE MILE OF A COASTAL EDGE. APPLY DESIGN LOAD REDUCTION VALUES FOR METAL-PLATE-CONNECTORS AND OTHER FASTENERS AS REQUIRED.
- 9. PERMANENTLY SUPPORT ALL CHORD AND WEB MEMBERS WITH CONTINUOUS LATERAL RESTRAINTS IN ACCORDANCE WITH INDUSTRY STANDARD DETAILS OR BCSI AT MEMBER LOCATIONS SPECIFIED BY THE TRUSS ENGINEER ON THE TRUSS SHOP DRAWINGS. FLOOR TRUSSES SHALL HAVE STRONGBACK BRIDGING AS SPECIFIED ON THE SHOP DRAWINGS. PROVIDE RESTRAINT DETAILS WITH SHOP DRAWING PACKAGE.
- IO. ALL PREFABRICATED FRAMING-TO-SIMILAR FRAMING
  CONNECTIONS SHALL BE DESIGNED BY THE MANUFACTURER OR
  SUPPLIER FOR THE CONCENTRATED LOADS OR REACTIONS
  INDICATED ON THE TRUSS SHOP DRAWINGS.
- II. ALL FLOOR JOIST AND TRUSS CONNECTIONS TO SUPPORTING STRUCTURAL BEAMS (LVL) SHALL BE CONNECTED WITH SIMPSON STRONG-TIE PRODUCTS FOR THE MAXIMUM REACTIONS INDICATED ON THE SHOP DRAWINGS. INSTALL SIMPSON PRODUCTS IN STRICT ACCORDANCE WITH MANUFACTURER SPECIFICATIONS AND 'GENERAL INSTRUCTIONS FOR THE INSTALLER' PROVIDED BY SIMPSON STRONG-TIE.
- 12. DESIGN OF METAL-PLATE-CONNECTED WOOD TRUSSES, FLOOR JOISTS AND SUPPORTING STRUCTURAL BEAMS (LVL) ARE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER OR JOIST SUPPLIER. REFER TO SHOP DRAWINGS AND FRAMING PLANS FOR FRAMING MEMBER DESIGNS.
- I3. CONCENTRATED LOADS ARE INDICATED ON PLAN AS  $(X^k)$  AND ARE ALLOWABLE STRESS DESIGN (ASD) VALUES.

## KEY NOTES

- CONTINUOUS ENGINEERED RIM BOARD. REFER TO 'TYPICAL FLOOR JOIST BEARING SECTIONS' DETAIL.
- PREFABRICATED FLOOR JOISTS OR TRUSSES.
  REFER TO SHOP DRAWINGS AND FLOOR LAYOUT.
- 3 2x4 EXTERIOR BEARING CMU WALL
- 4 (3) 2×10 GIRDER
- 5 CMU PIER, SEE TYPICAL DETAIL
- 6 PT 2X8 DECK JOISTS 16" OC
- 7 PT (2) 2XIO



FIRST FLOOR FRAMING PLAN

ering Services
SIGN, LLC
SHINGTON STREET, SUITE 2
VIRGINIA 23434
7.809.3700

SIMPLE DESIGN 521 W. WASHIN SUFFOLK, VIRG PHONE:757.809

FLOOR FRAMING PLAN

INKBEI

ERRY COURT

LOUIS RHETT CRIBB
Lic. No. 0402065775

8/14/2023

PROJ NO. 2876

DATE: 8/14/2023

SCALE: AS SHOWN

CHECKED: GSF

DRAWN BY: LRC

SF101

SHEET 5 OF 10

KEY NOTES

| | 1/8" LVL HEADER

2 (2) 7 1/4" LVL HEADER

5 (3) 18" LVL RIDGE BEAM

6 3.5" X 7" EWP POST

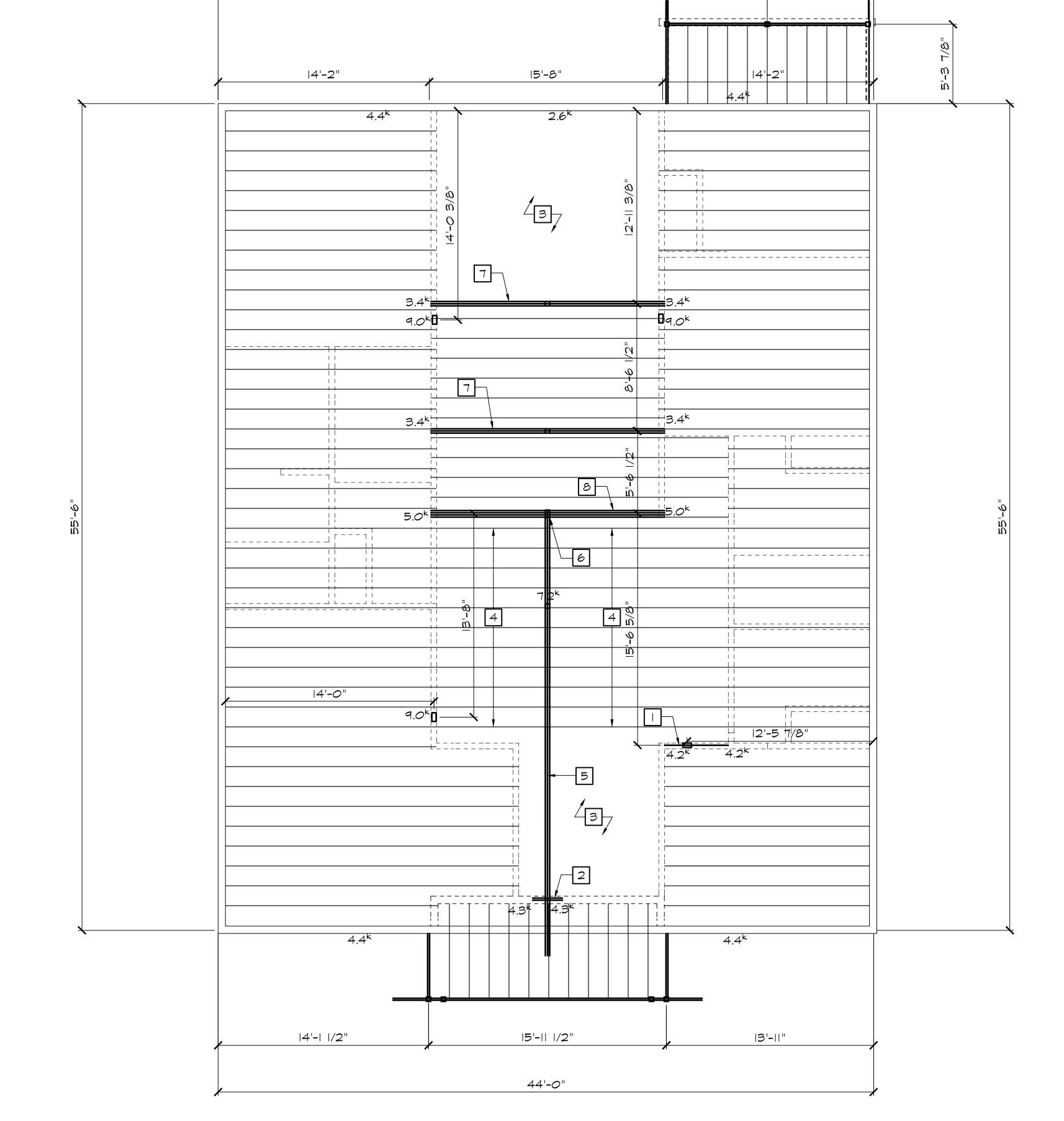
7 (3) || 7/8" LVL BEAM

8 (4) || 7/8" LVL BEAM

3 VAULTED CEILING IS FRAMED BY RAFTERS, SEE ROOF FRAMING PLAN

4 MAINTAIN SLOPE OF VAULTED CEILING WITH CEILING JOISTS TYING INTO RIDGE BEAM

- I. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT NOTED.
- 2. CEILING JOISTS, UNLESS OTHERWISE NOTED, SHALL BE 2XIO JOISTS SPACED AT 16" OC. ALL JOISTS EXPOSED TO OUTDOOR CONDITIONS SHALL BE PRESERVATIVE TREATED.
- 3. CONCENTRATED LOADS ARE INDICATED AS  $(X^k)$  AND ARE ALLOWABLE STRESS (ASD) DESIGN VALUES



44'-0"

6'-8 3/4"

6'-9 1/4"

CEILING FRAMING PI

SHEET 6 OF 10

SF102

SCALE: AS SHOWN

CHECKED: DRAWN BY:

8/14/2023

LOUIS RHETT CRIBB Lic. No. 0402065775

DATE

REVISION

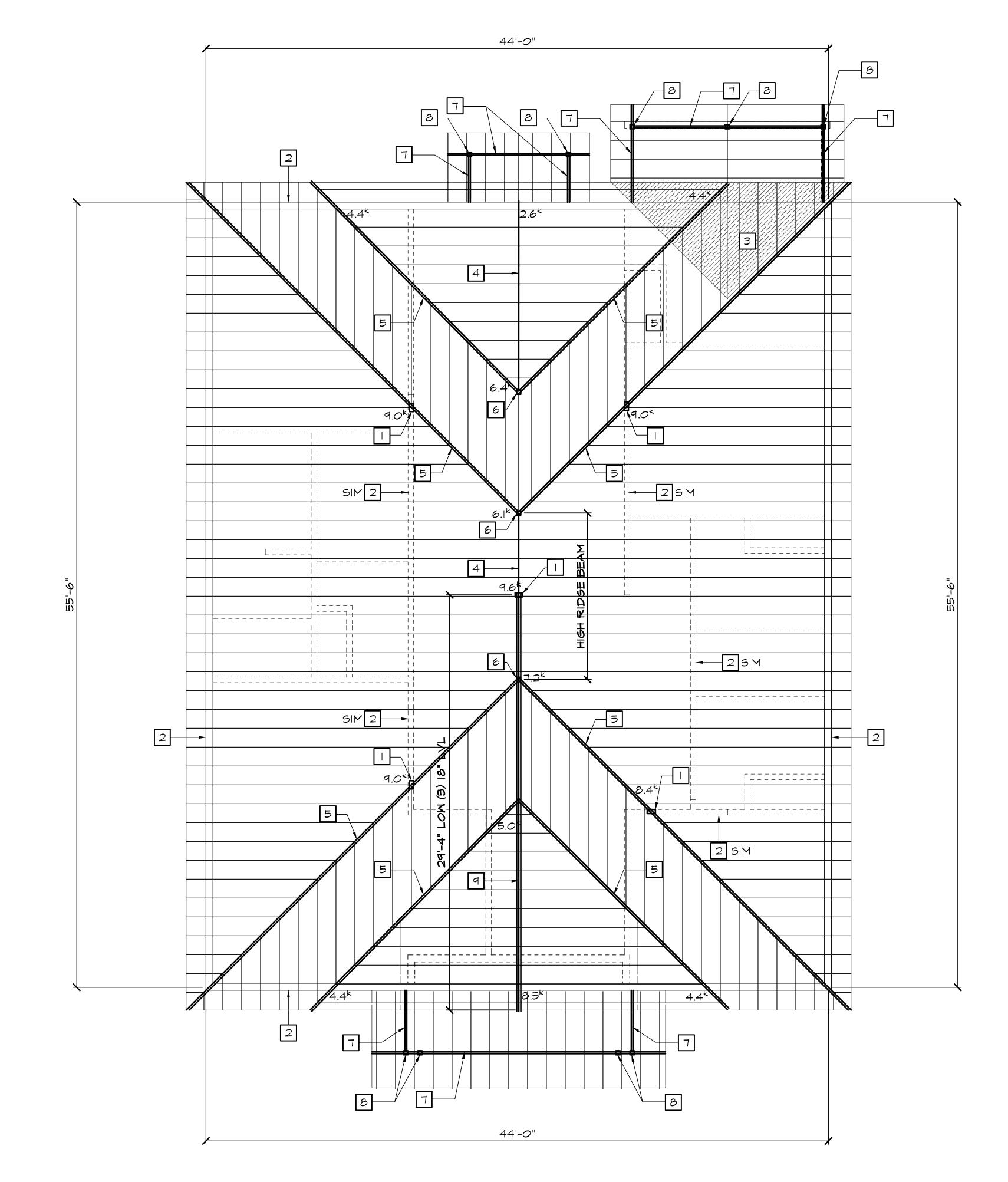
INKBERRY

RAMING F

CEILING FRAMING PLAN

SCALE: 3/16" = 1'-0"

- I. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT NOTED.
- 2. ROOF CONSTRUCTION, UNLESS OTHERWISE NOTED, SHALL BE 2XIO RAFTERS SPACED AT 16" OC.
- 3. CONCENTRATED LOADS ARE INDICATED AS  $(X^k)$  AND ARE ALLOWABLE STRESS (ASD) DESIGN VALUES



# KEY NOTES

- 3.5" X 7" EWP POST
- 2 2x4 EXTERIOR LOAD BEARING WALL, 2X4 INTERIOR LOAD BEARING WALL AT SIM
- 3 OVER FRAMING, REFER TO "TYPICAL VALLEY AND OVER FRAMING DETAIL"
- 4 | 1 7/8" LVL RIDGE BEAM
- 5 (2) || 7/8" LVL HIP/VALLEY BEAM
- 6 3.5" X 3.5" EWP POST
- 7 PT (2) 2XIO
- 8 PT 4X4 POST
- 9 (3) 18" LVL RIDGE BEAM



SCALE: 3/16" = 1'-0"

Engineering ServisimPLE DESIGN, LLC 521 W. WASHINGTON STRISUFFOLK, VIRGINIA 23434 PHONE:757.809.3700

REVISION DATE

INKBERRY

ERRY COURT

LOUIS RHETT CRIBB
Lic. No. 0402065775

8/14/2023

PROJ NO. 2876

DATE: 8/14/2023

SCALE: AS SHOWN

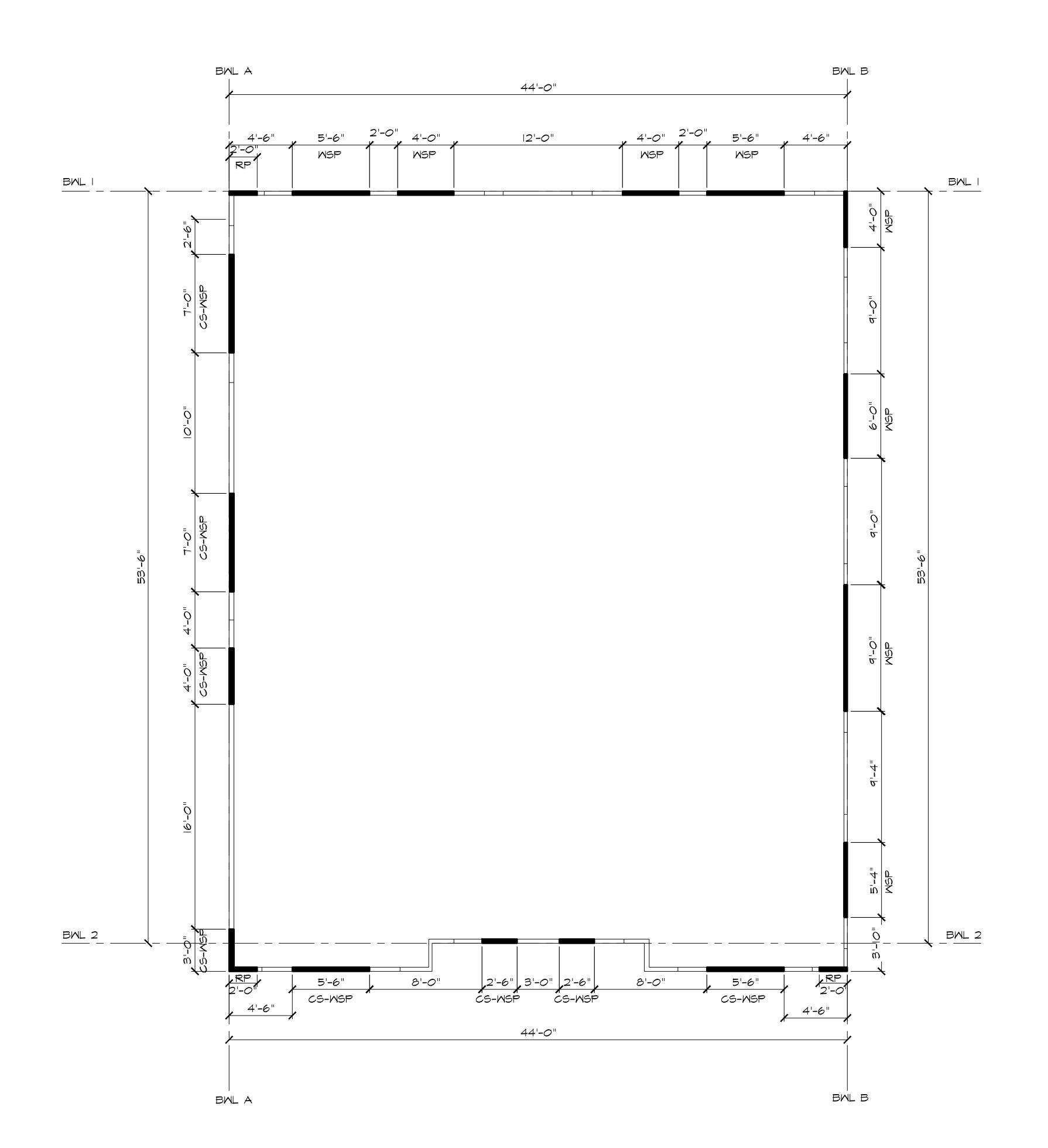
CHECKED: GSF

DRAWN BY: LRC

SHEET 6 OF 10

SF103

- I. COORDINATE SCHEDULES ON SF602 WITH SHEARWALL AND BRACED WALL PANELS SHOWN ON PLANS.
- 2. HOLD DOWN DEVICES REQUIRED AT END OF SHEARWALLS AND AT ENDS OF BRACED WALL PANELS AT LOCATIONS INDICATED.
- 3. HOLD DOWN DEVICE (HD) AT ENDS OF CS-MSP PANELS AND PFH PORTAL FRAMES SHALL BE SIMPSON MSTC52 STRAP TIE.
- 4. HOLD DOWN DEVICE (HD) AT ENDS OF SHEARWALLS (SW) SHALL BE SIMPSON MSTC52 STRAP TIE.
- 5. REFER TO PARTIAL PLANS FOR OTHER REQUIRED BMP AND SM LOCATIONS AND HOLD DOWN LOCATIONS.
- 6. ALL 2x12 HEADERS OVER ROUGH OPENINGS GREATER THAN 7'-6" SHALL BE SUPPORTED INTERMITTENTLY WITH GANG STUD COLUMNS LOCATED BETWEEN WINDOWS.



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REVISION	DATE

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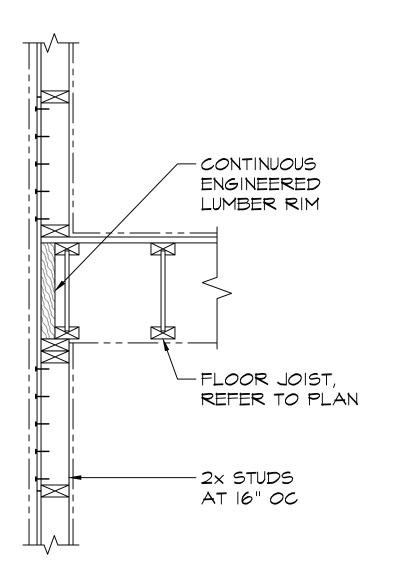
8/14/2023 AS SHOWN DRAWN BY:

SF104

FIRST FLOOR BRACED WALL PLAN

SCALE: 1/4" = 1'-0"

#### EXTEND SHEATHING CONTINUOUS -BLOCKING AT JOINTS AS 16" MIN ABOVE SOLE PLATE AND REQUIRED FOR SHEARWALLS 16" MIN BELOW DOUBLE TOP PLATE AND FASTEN TO EACH -CONTINUOUS SOLE PLATE STUD WITH (5) 8d NAILS. (AS AN FASTENED TO RIM WITH ALTERNATE: LOCATE JOINTS 10d NAILS AT 6" OC FOR FULL SHEETS, 4'-0" MIN, IN -3/4" T&G WOOD PANEL THE MIDDLE THIRD OF THE RIM SUBFLOOR GLUED JOIST AND FASTEN WITH AND FASTENED WITH 2 ROWS OF 8d NAILS AT 3" OC 10d NAILS AT 6" OC ON BOTH SIDES OF JOINT) CONTINUOUS ENGINEERED LUMBER RIM. FASTEN TO DOUBLE TOP PLATE WITH 6d TOE NAILS AT 4" OC, STAGGERED INSIDE AND OUTSIDE -FLOOR JOIST, REFER TO PLAN DOUBLE TOP PLATE 1/2" NOMINAL PLYWOOD -2x STUDS AT 16" 00 SHEATHING. REFER TO SHEARWALL SCHEDULE FOR ADDITIONAL REQUIREMENTS.



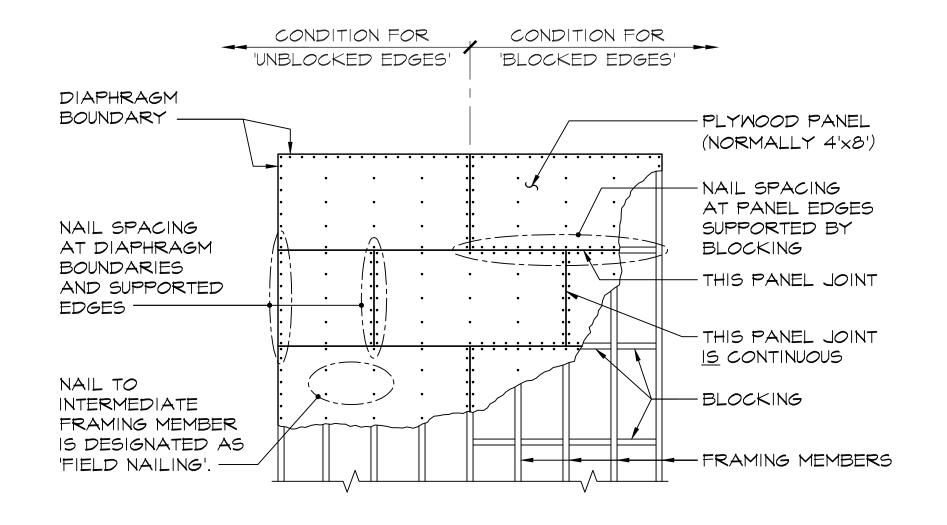
### PARALLEL

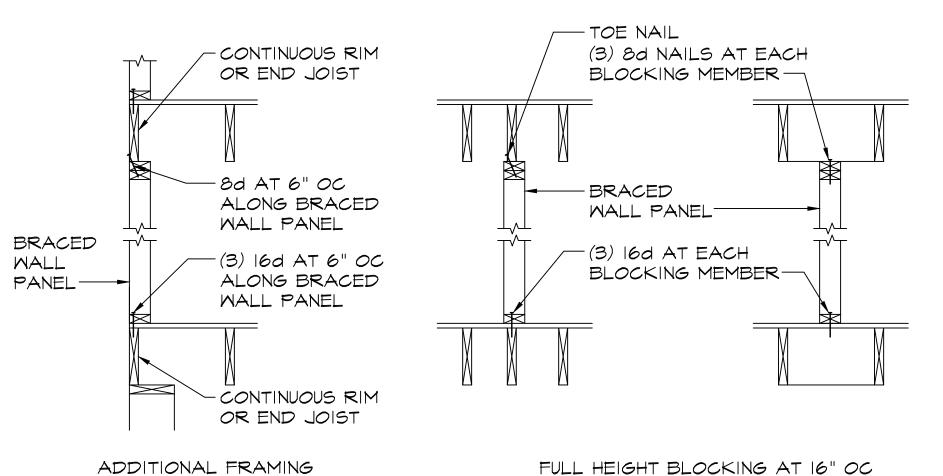
# TYPICAL RAFTER BEARING SECTION

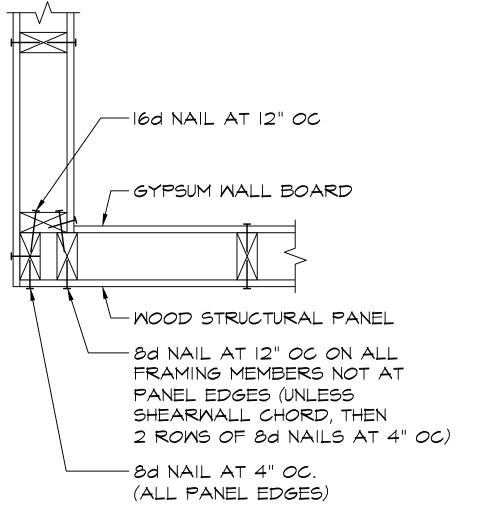
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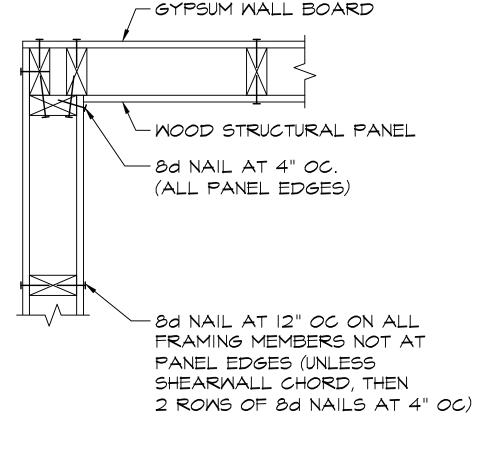
TYPICAL FLOOR JOIST BEARING SECTIONS

PERPENDICULAR









INSIDE CORNER

TYPICAL PLYWOOD DIAPHRAGM DETAIL NOT TO SCALE

TYPICAL VALLEY FRAMING DETAIL

NOT TO SCALE

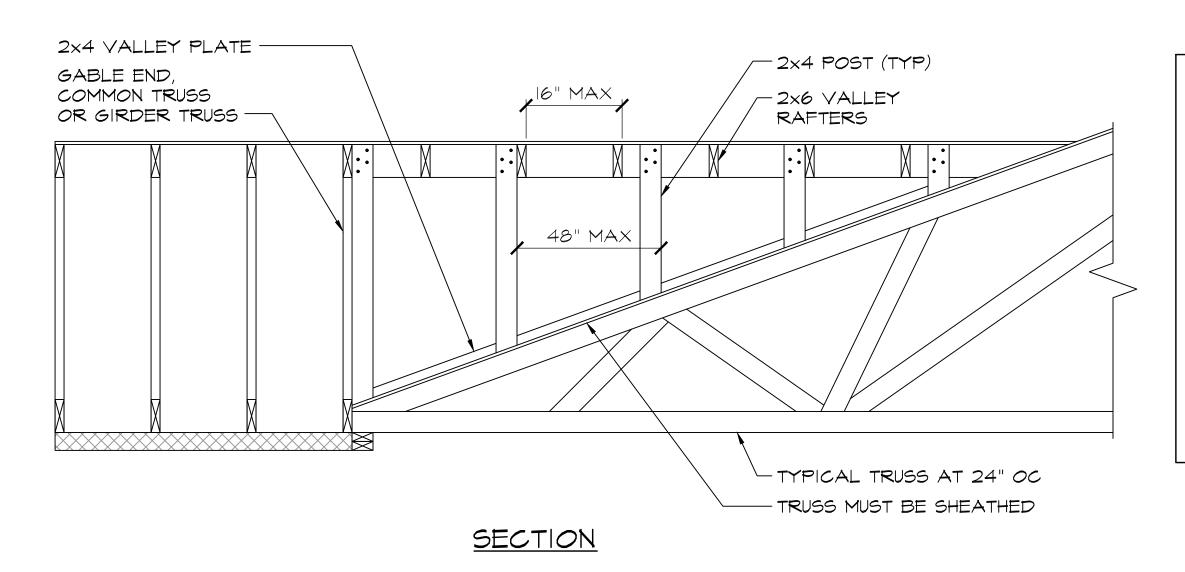
BRACED WALL PANEL CONNECTION WHEN PARALLEL TO FLOOR/CEILING FRAMING NOT TO SCALE

MEMBER DIRECTLY BELOW

BRACED WALL PANEL

OUTSIDE CORNER

# CORNER FRAMING DETAILS NOT TO SCALE

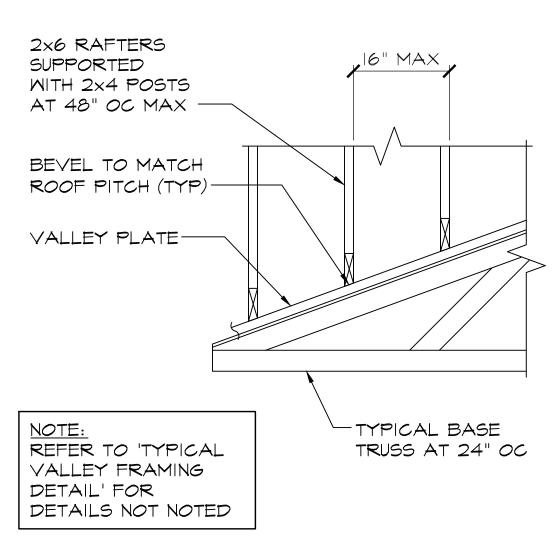


## VALLEY FRAMING NOTES:

WITH BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TO TOP CHORD OF SUPPORTING (BASE) TRUSSES

ALONG BRACED WALL PANEL

- 2. BRACE BOTTOM CHORD AND WEB MEMBERS PER TRUSS DESIGNS.
- 3. INSTALL 2x4 VALLEY PLATES. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5"x.|3|") NAILS.
- 4. SET 2x6 #2 RIDGE BOARD. SUPPORT WITH 2x4 POSTS SPACED AT 48" OC. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) IOd (3"x.I3I") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) IOd (3"x.I3I") TOE-NAILS.
- 5. FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 16" OC. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5"x.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5"x.131") TOE-NAILS.
- 6. SUPPORT THE VALLEY RAFTERS WITH 2x4 POSTS AT 48" OC (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) IOd (3"x.I31") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5"x.131") NAILS.



TYPICAL OVER FRAMING DETAIL NOT TO SCALE

DRAWN BY: SF501

8/14/2023

NONE

LOUIS RHETT CRIBB

Lic. No. 0402065775

PROJ NO.

SCALE:

CHECKED:

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SHEET 9 OF 10

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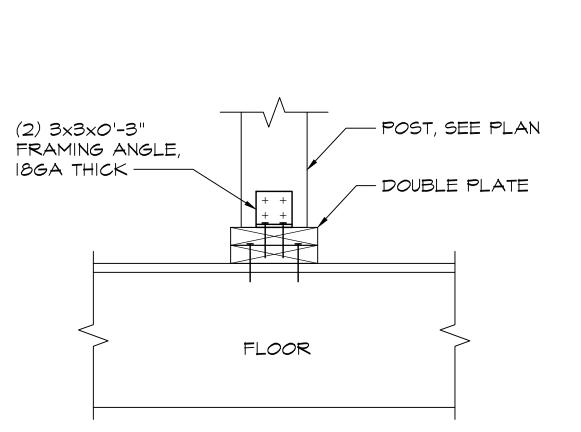
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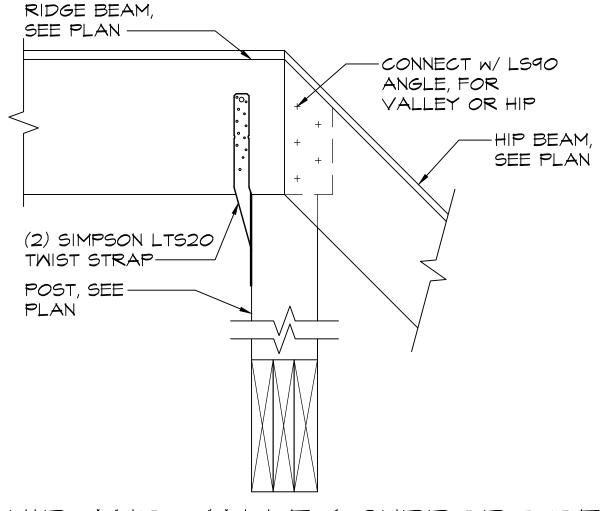
23310

GANG STUD COLUMN SCHEDULE							
	8' TALI	_ MALL	9' TALI	_ MALL	10' TALL MALL		
LOAD	2x4 STUD WALL	2×6 STUD WALL	2×4 STUD WALL	2×6 STUD WALL	2×4 STUD WALL	2×6 STUD WALL	
3.0K	(2) 2×4	(2) 2×6	(3) 2×4	(2) 2×6	(3) 2×4	(2) 2×6	
4.5K	(3) 2×4	(2) 2×6	(4) 2×4	(2) 2×6	(4) 2×4	(2) 2×6	
6.0K	(4) 2×4	(2) 2×6	3.5"x3.5" EMP COLUMN	(2) 2×6	3.5"×5.5" EMP COLUMN	(2) 2×6	
7.5K	3.5"×3.5" EMP COLUMN	(2) 2×6	3.5"X5.5" EMP COLUMN	(3) 2×6	SEE PLANS	(3) 2×6	
IOK	3.5"X7" EMP COLUMN	(3) 2×6	3.5"X7" EMP COLUMN	(4) 2×6	SEE PLANS	(3) 2×6	
>IOK	SEE PLANS	SEE PLANS	SEE PLANS	SEE PLANS	SEE PLANS	SEE PLANS	

#### GANG STUD SCHEDULE NOTES:

- I. SCHEDULE IS FOR GANG STUDS WITHIN A WALL. REFER TO PLANS FOR INDIVIDUAL COLUMN SIZES.
- 2. SIZES SHOWN ARE MINIMUM NUMBER OF STUDS. TOTAL COLUMN WIDTH SHOULD PROVIDE FULL BEARING UNDER SUPPORTED MEMBER.





POST TO FLOOR DETAIL NOT TO SCALE

INTERRUPTED

HEADER, REFER

TO SCHEDULE

FULL HEIGHT STUDS

SEE SCHEDULE

JACK STUDS

SEE SCHEDULE

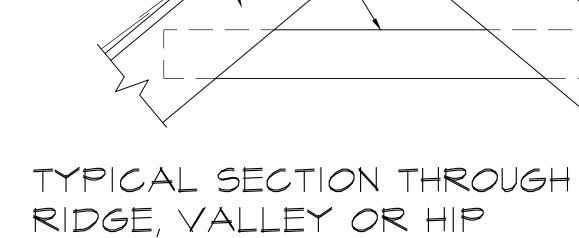
ROUGH OPENING

SEE ARCH DWGS

TYPICAL OPENING

STUDS -

HIP AND VALLEY SUPPORT DETAIL NOT TO SCALE



-(4) 16d TOE NAILS

(3 1/2"x0.135")

EACH SIDE

# MOOD HEADER SCHEDULE, 2x4 WALLS

ROUGH	COMPOSITION	JACK	STUDS	FULL HEIGHT	REMARKS	
OPENING		IST FLOOR	2ND FLOOR	STUDS		
0 TO 4'-0"	(2) 2×8	2		2		
4'-1" TO 6'-0"	(2) 2×10	2	2	3		
6'-1" TO 7'-6"	(2) 2×12	2	2	4		
7'-7" TO 9'-0"	(2)   3/4"×  7/8" LVL	3	2	5		

\* REFER TO PLAN FOR HEADER SIZE, IF NOT SHOWN USE SCHEDULES

MOOD	HEADER	SCHEDULE,	2×6	MALLS

			•		
ROUGH	COMPOSITION	JACK	STUDS	FULL HEIGHT	REMARKS
OPENING		IST FLOOR	2ND FLOOR	STUDS	
0 TO 5'-0"	(3) 2×8	2		2	
5'-1" TO 7'-6"	(3) 2×10	2	1	Э	
7'-7" TO 9'-0"	(3) 2×12	2	2	4	
9'-1" TO 11'-0"	(3)   3/4"×117/8" LVL	3	2	5	

#### WOOD HEADER SCHEDULE NOTES:

NOT TO SCALE

CONT. RIDGE BOARD

2×6 COLLAR TIES

RAFTER, REFER

TO ROOF PLAN-

AT 32" OC (R602.3(I) -

OR BEAM (AT RAFTERS),

REFER TO ROOF PLAN -

- I. HEADER SCHEDULE APPLIES TO MEMBERS IN PERIMETER AND INTERIOR BEARING WALLS NOT OTHERWISE NOTED ON DRAWINGS.
- 2. FULL HEIGHT STUDS APPLY TO EXTERIOR WALLS AND SHEARWALLS ONLY. PROVIDE SINGLE FULL HEIGHT STUD TO ALL OTHER WALLS.
- 3. WHERE SPECIFIED JACK STUDS AND FULL HEIGHT STUDS WILL NOT FIT WITHIN THE WALL, PROVIDE FRAMING ANCHORS CAPABLE OF SUPPORTING THE FULL REACTION OF THE HEADER, AND FRAME HEADER INTO THE SIDE OF THE FULL HEIGHT STUDS.
- 4. PROVIDE PLYWOOD FLITCH PLATES OR SPACERS AS REQUIRED.
- 5. FOR HEADERS AT LARGER OPENINGS AND HEADERS WITH SPECIAL LOADS, REFER TO PLAN FOR HEADER CONSTRUCTION.

	FIRST FL	_00	OR E	BRA	*CEI	> \rangle	ALL		NE	
			50	<u> </u>	DULE	= - =				
MIN	ND SPEED (MPH)		120	9	120	2	120		120	9
BR	ACED WALL LINE			I	2	2	f	4	E	3
AV	G. BWL SPACING (	FT)	53	.50	53.	.50	44.00		44.00	
TA	BULAR REQUIRED	(FT)	10.	20	8.9	53	7.	10	8.6	60
	EXPOSURE		C	1.20	C	1.20	C	1.20	C	1.20
	EAVE RIDGE HEIGHT (F	т)	17.40	1.44	17.40	1.44	17.40	1.44	17.40	1.44
Ä	STORY HEIGHT (FT)		9.0	0.95	9.0	0.95	9.0	0.95	9.0	0.95
ADJUSTMENT	#BMLS		2.0	1.00	2.0	1.00	2.0	1.00	2.0	1.00
Ď	OMIT INTERIOR GB		NO	1.0	NO	1.0	NO	1.0	NO	1.0
٩	ADD PAIR 800# HOLD DOWNS		NO	1.0	NO	1.0	NO	1.0	NO	1.0
	METHOD GB FASTEN AT 4" OC		NO	1.0	NO	1.0	NO	1.0	NO	1.0
RE	QUIRED BMP LENGT	H (FT)	16.	79	14.04 11.69		<del></del>	14.16		
		BMP	METHOD	LENGTH (FT)	METHOD	LENGTH (FT)	METHOD	LENGTH (FT)	METHOD	LENGTH (FT)
		I	WSP	5.50	CS-WSP	2.50	CS-WSP	3.00	MSP	5.33
0 <u>N</u>		2	WSP	5.50	CS-WSP	2.50	CS-WSP	4.00	MSP	9.00
	CONTRIBUTING	3	WSP	4.00	CS-WSP	5.50	CS-WSP	7.00	MSP	11.00
ACTUAL	LENGTH	4	MSP	4.00	CS-WSP	2.50	CS-WSP	7.00	MSP	15.25
A V		5								
		6								
		7								
AC	TUAL BWP LENGTH	(FT)	19.0	00	16.0	00	20	).I3	17.0	00

HURRICANE CLIP/TIE SCHEDULE					
UPLIFT LOAD (LBS)	SIMPSON STRONG TIE HURRICANE CLIP/TIE				
1015 MAX	HIOA (W/ 9-10d x 1 1/2")				
1450 MAX	HTS20 (w/ 20-10d x 1 1/2")				
2050 MAX	LGT2 (W/ 14-16d SINKERS)				
2900 MAX	HTS20 DOUBLE				

TYPICAL WOOD HEADER DETAIL

NOT TO SCALE

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REVISI	ON		D	PΑ	Έ	

INKBERRY

AILS

Lic. No. 0402065775

PROJ NO.	2876
DATE:	8/14/2023
SCALE:	NONE
CHECKED:	GSF
DRAWN BY:	LRC

SF601

SHEET 10 OF 10